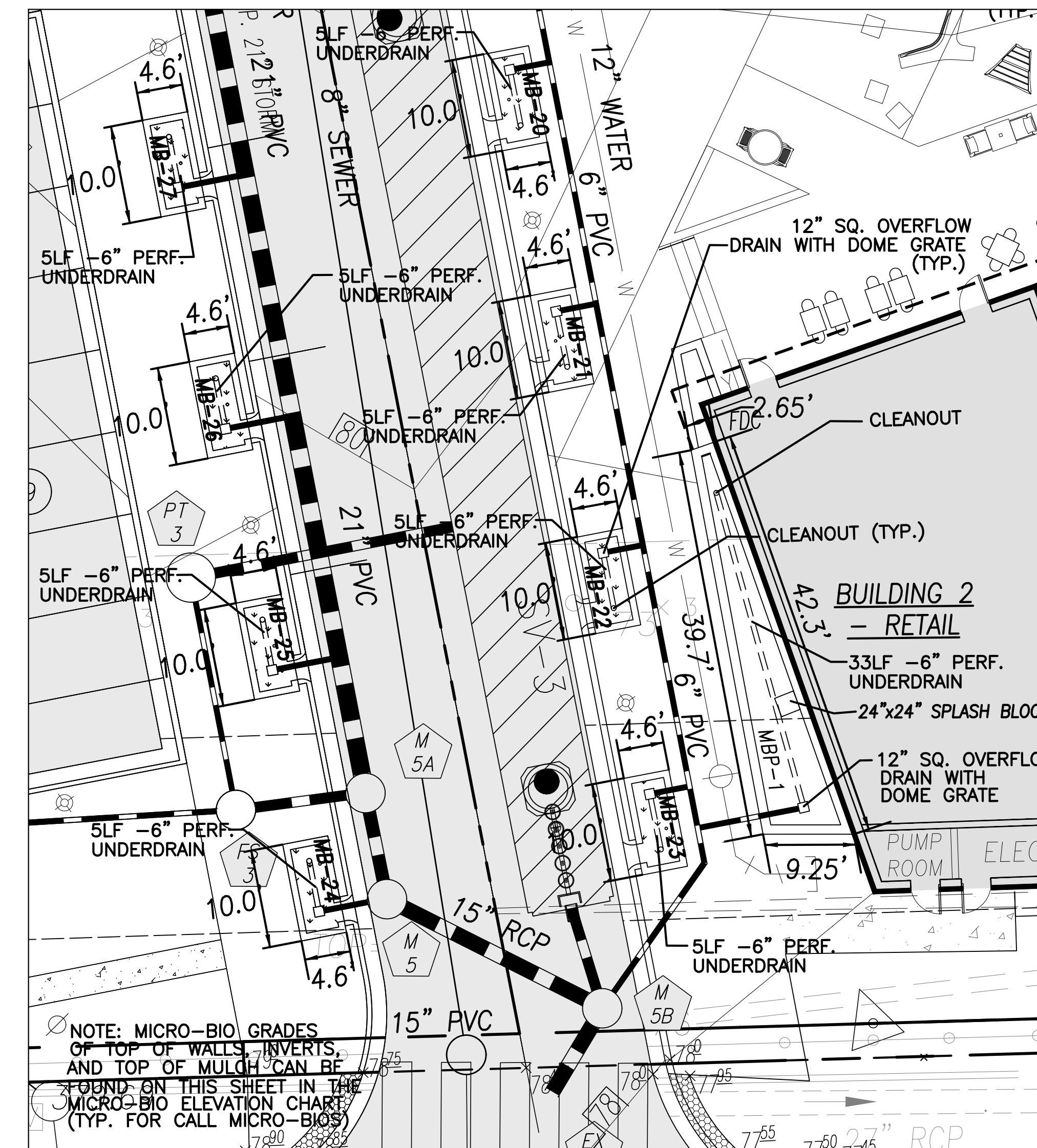
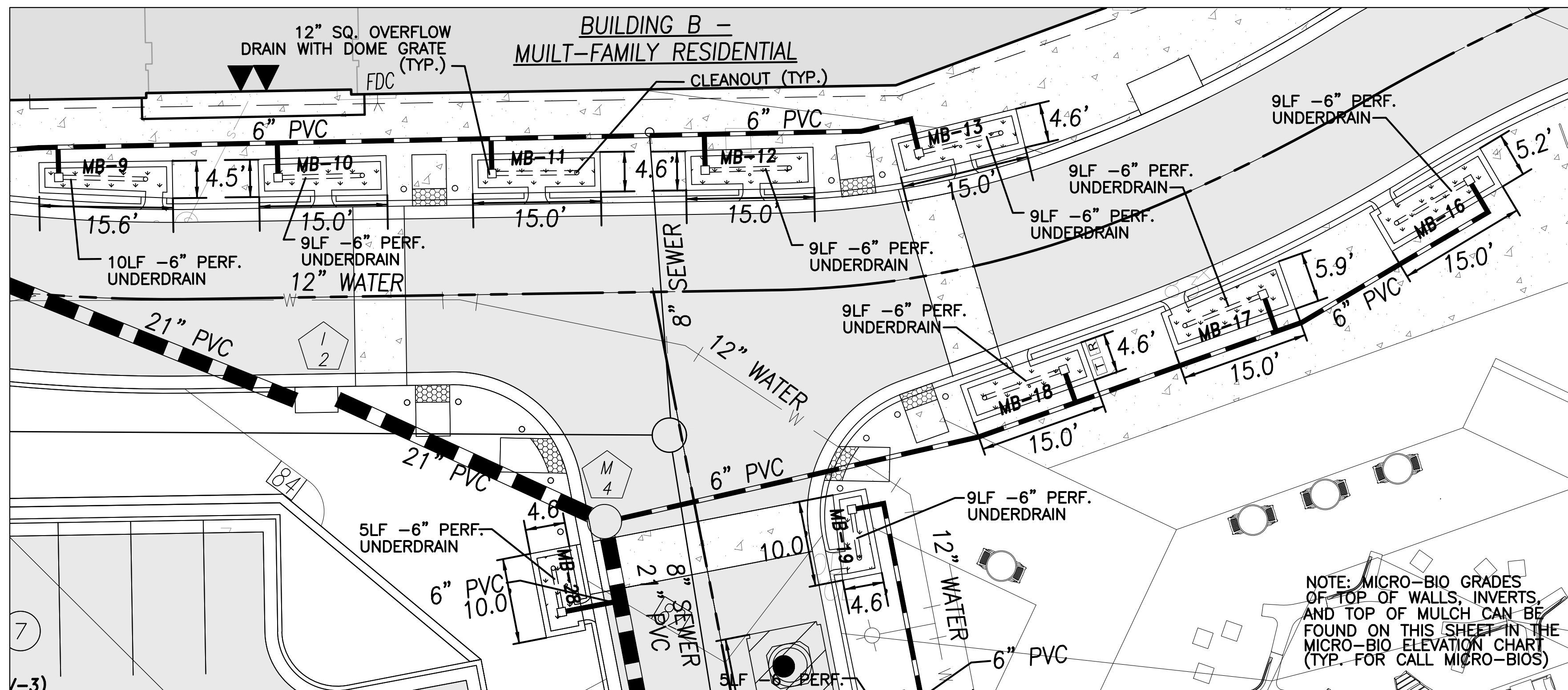
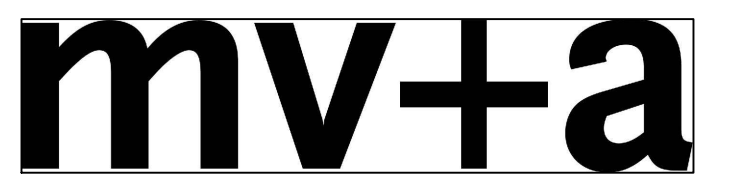


MB#	Inflow Elevation (Gutter)	Adjacent Road Slope (%)	Surface Elevation	Top of Cleanout (cap)	Overflow Structural Ponding Elevation	Underdrain Invert	Top of Wall Elevation (Low)	Top of Wall Elevation (High)	10 Year WSEL
MB-1	91.64 ft	4.8%	90.64 ft	91.14 ft	91.14 ft	86.14 ft	92.14 ft	93.05 ft	91.00 ft
MB-2	90.48 ft	4.8%	89.48 ft	89.98 ft	89.98 ft	85.48 ft	90.98 ft	91.70 ft	90.03 ft
MB-3	89.32 ft	4.8%	88.32 ft	88.82 ft	88.82 ft	84.32 ft	89.32 ft	90.54 ft	88.87 ft
MB-4	88.40 ft	4.8%	87.40 ft	87.90 ft	87.90 ft	83.40 ft	88.90 ft	89.33 ft	87.95 ft
MB-7	84.75 ft	4.8%	83.75 ft	84.25 ft	84.25 ft	78.50 ft	85.25 ft	86.26 ft	84.36 ft
MB-9	83.65 ft	2.0%	82.65 ft	83.15 ft	83.15 ft	77.40 ft	84.15 ft	84.45 ft	83.23 ft
MB-10	83.25 ft	0.0%	82.25 ft	82.75 ft	82.75 ft	78.25 ft	83.75 ft	83.75 ft	82.80 ft
MB-11	83.75 ft	0.0%	82.75 ft	83.25 ft	83.25 ft	78.75 ft	84.25 ft	84.25 ft	83.32 ft
MB-12	83.78 ft	0.0%	82.78 ft	83.28 ft	83.28 ft	78.78 ft	84.28 ft	84.28 ft	83.34 ft
MB-13	83.30 ft	0.0%	82.30 ft	82.80 ft	82.80 ft	76.55 ft	83.80 ft	83.80 ft	82.89 ft
MB-16	83.84 ft	1.5%	82.84 ft	83.84 ft	83.84 ft	77.09 ft	84.34 ft	84.57 ft	83.96 ft
MB-17	83.50 ft	1.5%	82.50 ft	83.50 ft	83.50 ft	76.75 ft	84.00 ft	84.23 ft	83.62 ft
MB-18	83.20 ft	0.0%	82.20 ft	82.70 ft	82.70 ft	78.20 ft	83.70 ft	83.70 ft	82.76 ft
MB-19	81.84 ft	4.1%	80.84 ft	81.34 ft	81.34 ft	75.09 ft	82.34 ft	82.75 ft	81.41 ft
MB-20	80.84 ft	4.1%	79.84 ft	80.34 ft	80.34 ft	74.59 ft	81.34 ft	81.75 ft	80.40 ft
MB-21	79.84 ft	4.1%	78.84 ft	79.34 ft	79.34 ft	73.59 ft	80.34 ft	80.75 ft	79.40 ft
MB-22	78.84 ft	4.1%	77.84 ft	78.34 ft	78.34 ft	72.59 ft	79.34 ft	79.75 ft	78.40 ft
MB-23	77.84 ft	4.1%	76.84 ft	77.34 ft	77.34 ft	71.59 ft	78.34 ft	78.75 ft	77.40 ft
MB-24	77.84 ft	4.1%	76.84 ft	77.34 ft	77.34 ft	71.59 ft	78.34 ft	78.75 ft	77.40 ft
MB-25	78.84 ft	4.1%	77.84 ft	78.34 ft	78.34 ft	72.59 ft	79.34 ft	79.75 ft	78.40 ft
MB-26	79.84 ft	4.1%	78.84 ft	79.34 ft	79.34 ft	73.59 ft	80.34 ft	80.75 ft	79.40 ft
MB-27	80.84 ft	4.1%	79.84 ft	80.34 ft	80.34 ft	74.59 ft	81.34 ft	81.75 ft	80.40 ft
MB-28	81.84 ft	4.1%	80.84 ft	81.34 ft	81.34 ft	75.09 ft	82.34 ft	82.75 ft	81.42 ft
MB-29	83.90 ft	4.8%	82.90 ft	83.40 ft	83.40 ft	77.65 ft	84.40 ft	85.12 ft	83.46 ft
MB-30	84.80 ft	4.8%	83.80 ft	84.30 ft	84.30 ft	78.55 ft	85.30 ft	86.02 ft	84.37 ft
MB-31	91.80 ft	4.8%	90.80 ft	91.30 ft	91.30 ft	85.05 ft	92.30 ft	92.78 ft	91.37 ft



- NOTE:
- 1.) ALL MICRO-BIO MATERIAL SPECIFICATION INFORMATION CAN BE FOUND ON SHEET C4.06.
 - 2.) ALL INVERTS FOR MICRO-BIO OVERFLOWS CAN BE FOUND ON SHEET C4.07.
 - 3.) ALL MICRO-BIO PLANTING INFORMATION CAN BE FOUND ON SHEET L104.
 - 4.) ALL STRUCTURAL INFORMATION FOR THE MICRO-BIO RETENTION PLANTERS CAN BE FOUND ON SHEETS S110 TO S111.



OWNER
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Gaithersburg, MD 20877
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LANDSCAPE ARCHITECT
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Tel: 703.549.7784

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Tel: 301.987.9234

MEP ENGINEER
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509 Germantown Pike Bldg 2
Lafayette Hill, PA 19444
Tel: 215.774.1429

North Bethesda Market II

NORTH BETHESDA, MARYLAND

**STORMWATER MANAGEMENT
DETAILS -
MICRO-BIORETENTIONS**

PROJECT NUMBER
22002.00

REV.	ISSUE:	DATE
1	Permit Set	10-22-24

KEY PLAN

DRAWING STAMP

Professional Certification
I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No.: 35287
Expiration Date: 01-07-26





- MICRO-BIORETENTION NOTES:**
- MULCH TO CONSIST OF STANDARD DOUBLE SHREDED AGED HARDWOOD. THE MULCH SHOULD BE APPLIED UNIFORMLY TO A DEPTH OF 3 INCHES. YEARLY REPLENISHING MAY BE NECESSARY. PINE BARK IS NOT ACCEPTABLE.
 - PLANTING MEDIA TO BE 1/3 PERLITE OR SOLITE, 1/3 COMPOST AND 1/3 SOIL.
(SEE SPECIFICATION BELOW)
 - SAND BED TO BE FINE AGGREGATE CONCRETE SAND PER ASTM C33 OR AASHTO M6.
(SEE SPECIFICATION BELOW)
 - THE GRAVEL LAYER SURROUNDING THE UNDERDRAIN PIPE MUST MEET MSHA SIZE #7 AND WELL DRAINING.
 - PERFORATIONS FOR UNDERDRAIN TO BE 3/8" DIAMETER HOLES SPACES 4" O.C., EVERY 90° AROUND PIPE.
 - GRATE DRAIN BASINS TO BE DOMED (TOP ELEVATIONS PROVIDED IS AT BASIN TOP, NOT TOP OF DOME).

- SAND SPECIFICATIONS:**
 WASHED ASTM C33 FINE AGGREGATE CONCRETE SAND IS UTILIZED FOR STORMWATER MANAGEMENT APPLICATIONS IN MONTGOMERY COUNTY, IN ADDITION TO THE ASTM C33 SPECIFICATION, SAND MUST MEET ALL OF THE FOLLOWING CONDITIONS:
- SAND MUST MEET GRADATION REQUIREMENTS FOR ASTM C-33 FINE AGGREGATE CONCRETE SAND.
 - AASHTO M-6 GRADATION IS ALSO ACCEPTABLE.
 - SAND MUST BE SILICA BASED, NO LIMESTONE BASED PRODUCTS MAY BE USED IF THE MATERIAL IS WHITE OR GRAY IN COLOR, IT IS PROBABLY NOT ACCEPTABLE. SAND MUST BE CLEAN.
 - NATURAL, UNWASHED SAND DEPOSITS MAY NOT BE USED. LIKEWISE, SAND THAT HAS BECOME CONTAMINATED BY IMPROPER STORAGE OR INSTALLATION PRACTICES WILL BE REJECTED.
 - MANUFACTURED SAND OR STONE DUST IS NOT ACCEPTABLE UNDER ANY CIRCUMSTANCES

- SOIL MEDIA SPECIFICATIONS:**
- THE PLANTING MEDIUM SHALL BE 24"-48" THICK AND SHALL CONSIST OF 1/3 PERLITE OR SOLITE, 1/3 COMPOST AND 1/3 TOPSOIL.
 - THE PERLITE SHALL BE COARSE GRADE HORTICULTURAL PERLITE.
 - THE COMPOST SHALL BE HIGH GRADE COMPOST FREE OF STONES AND PARTIALLY COMPOSTED WOODY MATERIAL.
 - THE TOPSOIL COMPONENT SHALL MEET THE FOLLOWING CRITERIA:
 - CONTAIN NO MORE THAN 10% CLAY
 - CONTAIN NO MORE THAN 10% TO 25% SILT
 - CONTAIN NO MORE THAN 60% TO 75% SAND
 - BE FREE OF STONES, STUMPS, ROOTS OR OTHER SIMILAR OBJECTS LARGER THAN 2 INCHES
 - THE FIRST LAYER OF THE PLANTING MEDIA SHALL BE LIGHTLY TILLED TO MIX IT INTO THE SAND 6" LAYER, SO AS NOT TO CREATE A DEFINITIVE BOUNDARY.
 - THE PLANTING BED SHALL BE FLOODED AFTER PLACEMENT.
 - ANY SETTLEMENT THAT OCCURS SHALL BE FILLED BACK TO THE DESIGN ELEVATION.

- GRAVEL BED SPECIFICATIONS:**
- THE GRAVEL LAYER SURROUNDING THE UNDERDRAIN PIPE(S) MUST MEET MSHA SIZE #7 (TABLE 901A), AND MUST PROVIDE A MINIMUM OF 6 INCHES COVER OVER THE PIPE(S), AND A MINIMUM 3 INCHES UNDER THE PIPE(S).
 - NO GEOTEXTILE OR FILTER FABRIC IS ALLOWED TO BE PLACED HORIZONTALLY ANYWHERE WITHIN THE FILTER MEDIA.
 - THE GRAVEL MUST BE CLEAN AND MUST BE STORED AND INSTALLED IN SUCH A MANNER THAT IT DOES NOT BECOME CONTAMINATED WITH SEDIMENT BEFORE OR AFTER INSTALLATION.

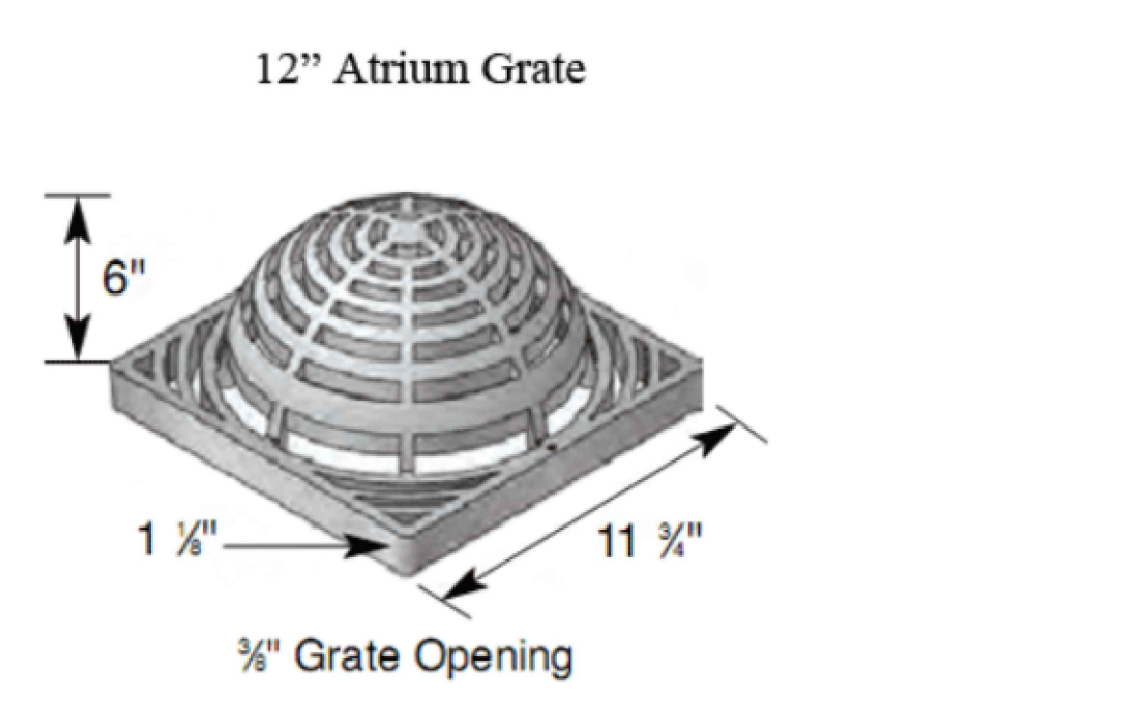
- UNDERDRAIN PIPE SPECIFICATIONS:**
- THE UNDERDRAIN PIPE CONSISTS OF 6-INCH DIAMETER SCHEDULE 40 OR STRONGER PERFORATED PVC PIPE AT 0.00% SLOPE.
 - THE UNDERDRAIN PIPE WILL BE PLACED WITHIN THE GRAVEL LAYER. A MINIMUM OF THREE (3) INCHES OF GRAVEL MUST BE PLACED UNDER THE PIPE, WITH A MINIMUM OF SIX (6) INCHES OF GRAVEL OVER THE PIPE.
 - PERFORATIONS MUST BE 3/8 INCH IN DIAMETER AND MUST BE LOCATED 4 INCHES ON CENTER, EVERY 90 DEGREES AROUND THE PIPE.
 - PERFORATED PIPE MUST BEGIN AT LEAST 12" INSIDE THE FILTER MEDIA
 - FILTER FABRIC MUST NOT BE WRAPPED AROUND THE UNDERDRAIN PIPE.

- NOTES:**
- Shop drawings for the CDS structures must be prepared, approved by the project engineer, and submitted to DPS plan reviewer for acceptance, utilizing standard precast checklist, prior to fabrication.
 - Annular space between pipe and hole to be filled with an approved non-shrink grout or concrete (as specified). For Stormceptor and Stormfilter, a rubber boot connection is required. Adapters are available for HDPE pipe.
 - Butyl rubber to be used in all joints. All joints to be grouted with non-shrink grout, inside and out
 - Concrete shall meet the requirements of ACI 350, Environmental Engineering Concrete Structures, with freezing and thawing exposures. Concrete shall be a Type 1L Cement conforming to ASTM C-595, with a 28-day Compressive strength of 5000 psi. Concrete shall also be in conformance with the latest edition and addenda of the MSHA Standards and Specifications for Construction and Materials

NDS
 12" CATCH BASIN
 Part #: 1200, 1203, 1207

Grate Number(s)	Description	Flow Rate with 1/2" Head
1210, 1211, 1212, 1215	12" x 12" Square Grate	155.28 GPM
1213	12" x 12" Square Cast Iron Grate	157.79 GPM
1215	12" x 12" Square Galvanized Steel Grate	143.80 GPM
1280, 1290	12" x 12" Atrium Grate	154.79 GPM
1218, 1219	12" Botanical Plastic	74.77 GPM
1218R, 1219R	12" Botanical Cast Iron	76.60 GPM
1224, 1224S	12" Wave Plastic	78.53 GPM
1224R, 1224R	12" Wave Cast Iron	78.31 GPM

Part #	Flow Rate per Outlet
1242, 1243	78.10
1245	Top: 78.09 GPM Middle: 76.30 GPM Bottom: 83.41 GPM
1266	247.12 GPM
1380	236.30 GPM

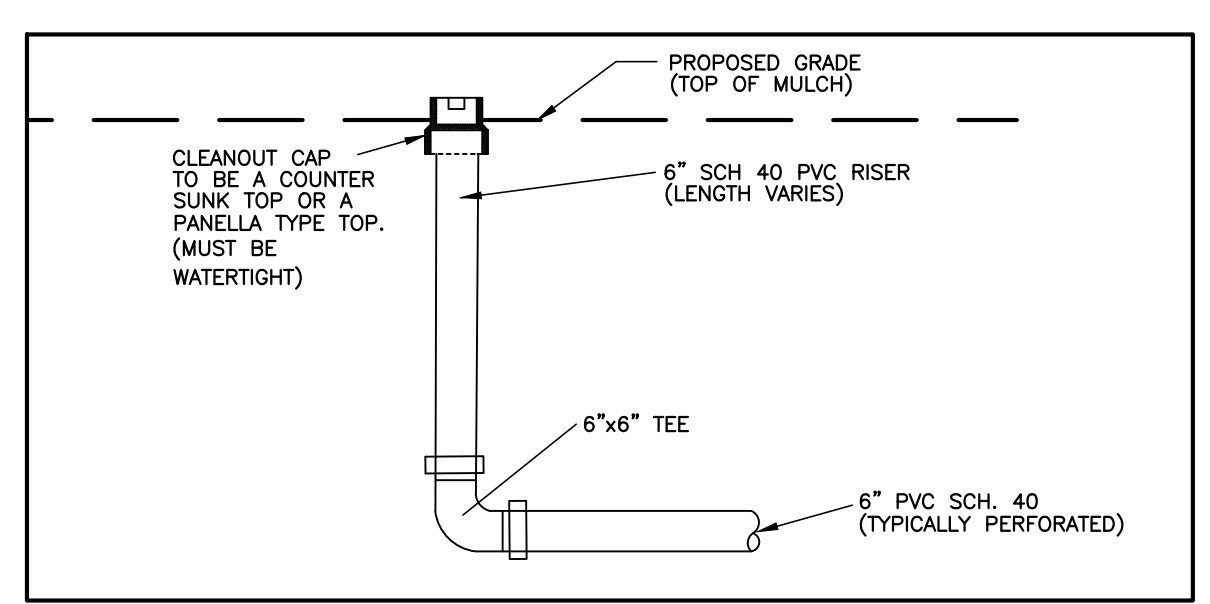


NDS
 12" Catch Basin Series

Model	Material	Flow Rate (GPM)	Notes
1200	Cast Iron	155.28	Standard
1203	Cast Iron	157.79	Standard
1207	Galvanized Steel	143.80	Standard
1280	Plastic	154.79	Standard
1290	Plastic	154.79	Standard
1218	Plastic	74.77	Standard
1219	Cast Iron	76.60	Standard
1224	Plastic	78.53	Standard
1224R	Cast Iron	78.31	Standard

NDS INLET CATCH BASIN RISER (FOR MICRO-BIO OVERFLOWS)

Model	Material	Flow Rate (GPM)
1200	Cast Iron	78.10
1203	Cast Iron	78.10
1207	Galvanized Steel	78.10



PRIVATE STORM STRUCTURE SCHEDULE

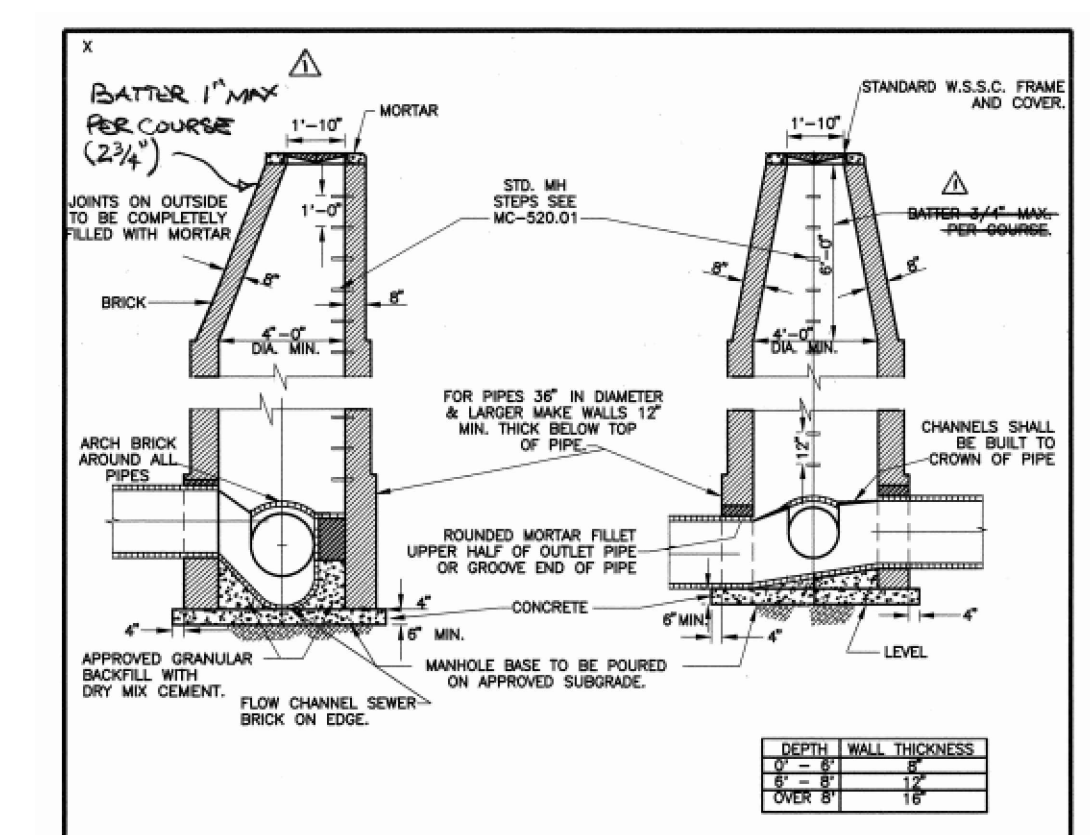
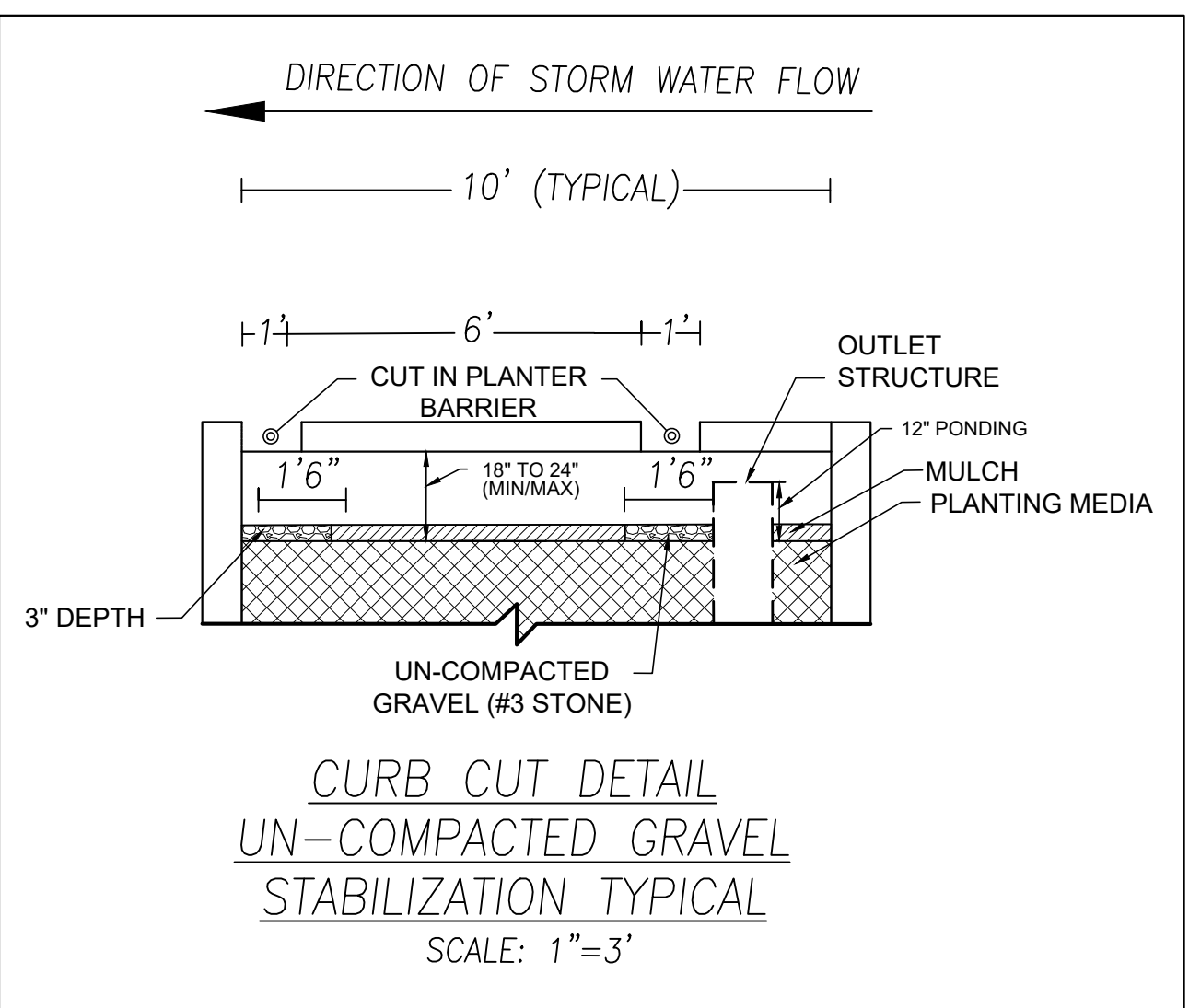
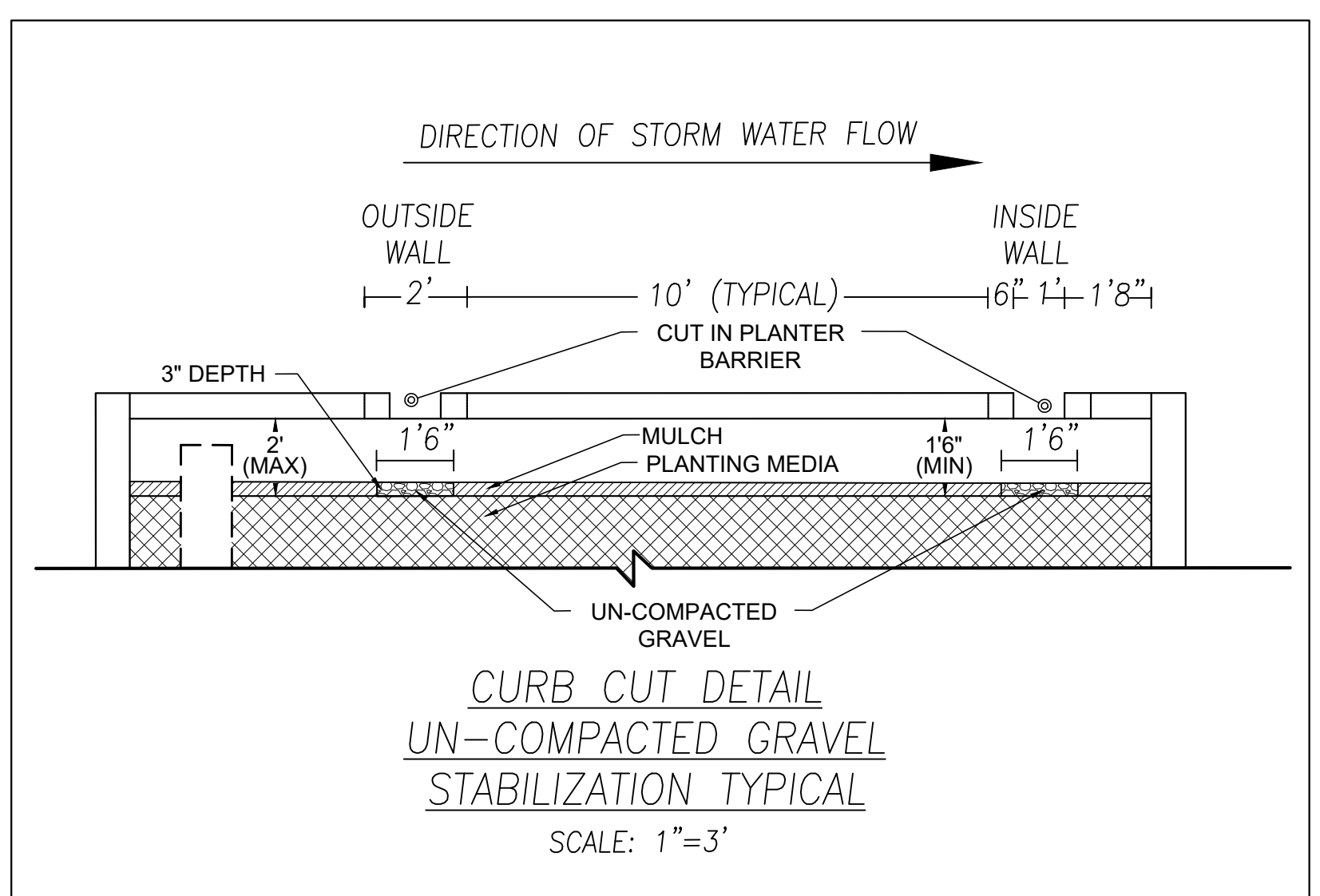
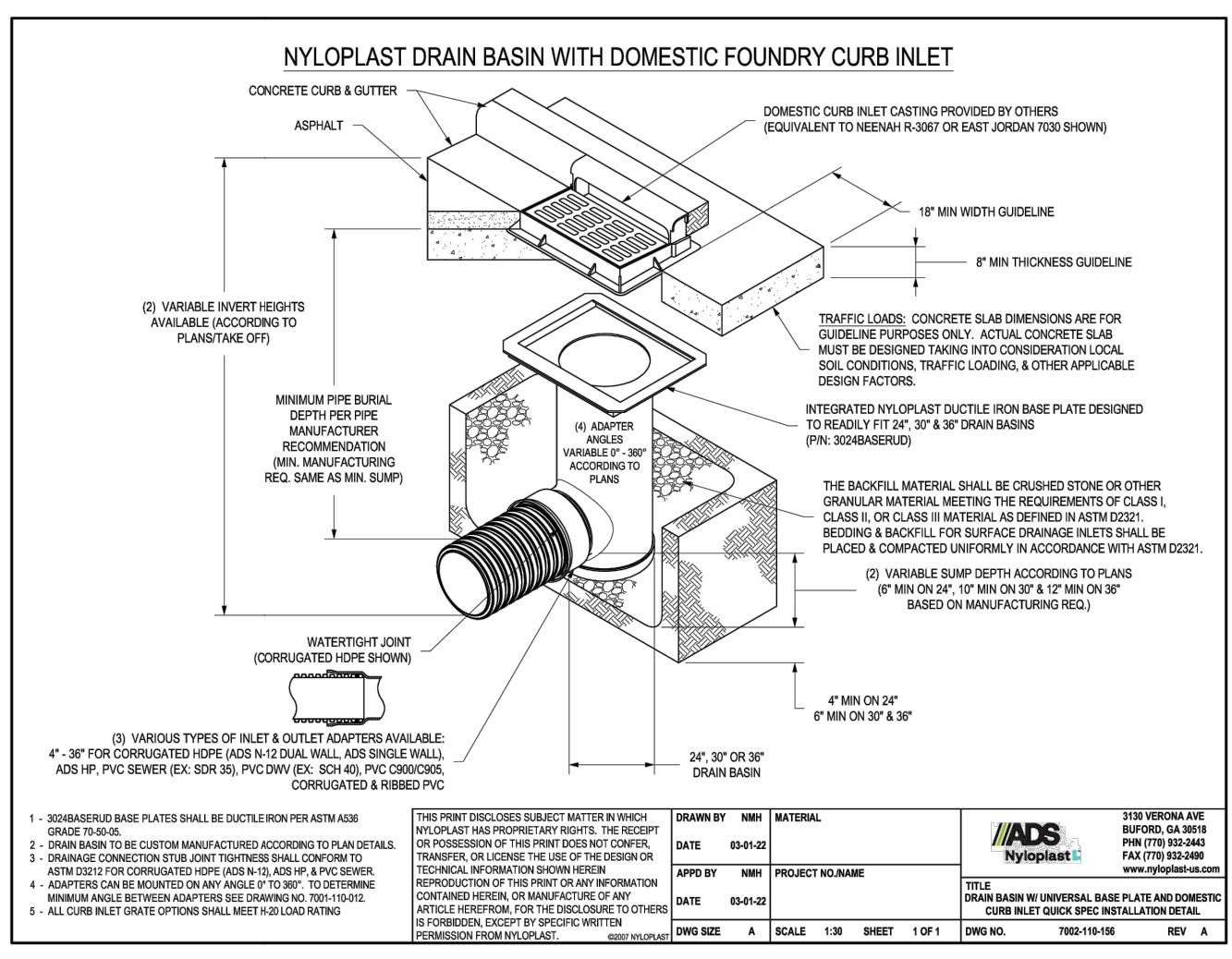
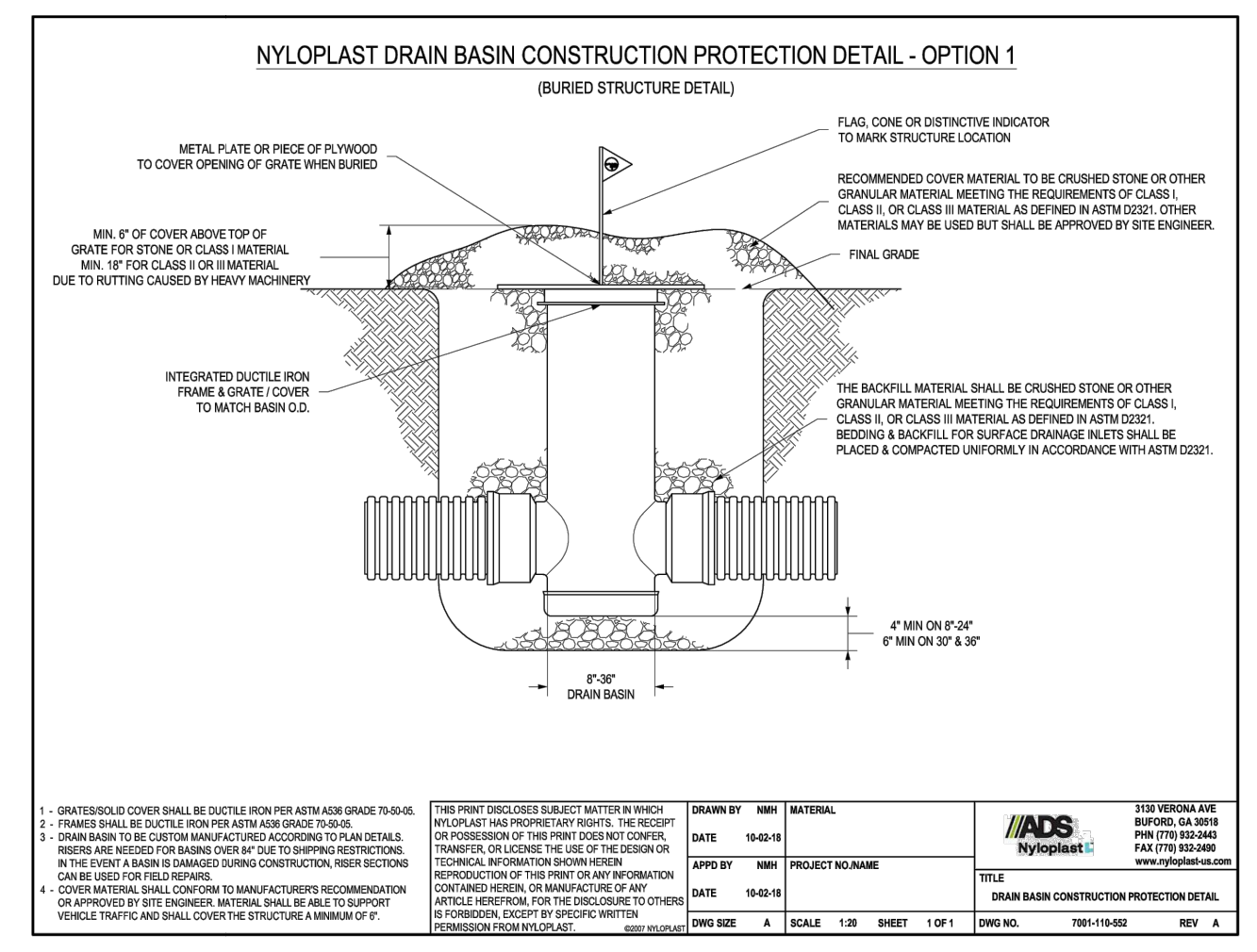
STR. NO.	DETAIL	TOP ELEV.	INV. IN	INV. IN	INV. OUT	REMARKS
M-1A	MC-510.01	386.40	373.50	373.83	373.40	TYPE 'A' 48" DIA. MH
M-1	MC-510.01	386.10	373.30	375.47	373.40	TYPE 'A' 48" DIA. MH
I-1	MD-374.14	385.85	380.13	376.25	376.15	Standard NR Inlet
M-2A	MC-510.01	383.77	378.85	370.95	370.70	TYPE 'A' 48" DIA. MH
M-2	MC-510.01	384.33	371.80	370.50	369.50	TYPE 'A' 48" DIA. MH
M-3	MC-510.01	383.87	369.45	369.10	368.35	TYPE 'A' 48" DIA. MH
I-2	MD-374.14	383.30	383.30	369.10	368.95	Standard NR Inlet
M-4	MC-510.01	382.50	368.55	368.55	368.45	TYPE 'A' 48" DIA. MH
M-5A	MC-510.01	379.10	368.00	372.10	367.90	TYPE 'A' 48" DIA. MH
M-5	MC-510.01	378.80	367.85	374.50	367.75	TYPE 'A' 48" DIA. MH
M-5B	MC-510.01	378.30	367.40	367.45	367.30	TYPE 'J' INLET
I-3	NDS Drain Basin	375.70	375.70	373.70	373.70	NDS - Curb Inlet

MBP STRUCTURE SCHEDULE - PRIVATE

STR. NO.	DETAIL	TOP ELEV.	INV. IN	INV. IN	INV. OUT	REMARKS
FS1	County Detail - Diversion Structure	387.33	383.10	381.17	382.50	Diversion Structure Manhole (48")
FS2	County Detail - Diversion Structure	384.33	380.25	378.00	379.00	Diversion Structure Manhole (48")
FS3	County Detail - Diversion Structure	379.60	373.20	371.17	372.17	Diversion Structure Manhole (48")
PT3	Inline CDS Standard Detail	379.80	371.07	371.07	371.07	Pre-Treatment Structure

MBP STRUCTURE SCHEDULE - PRIVATE

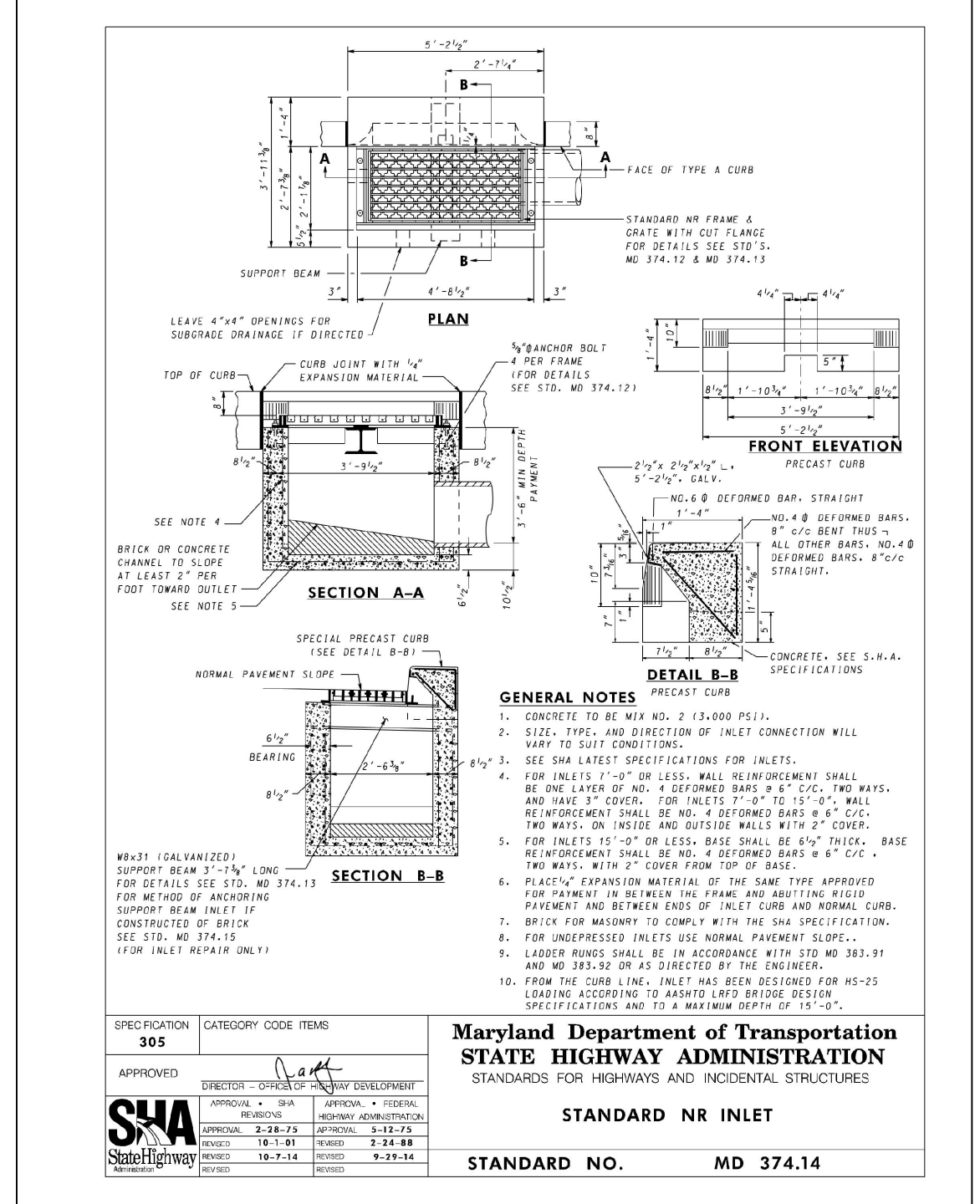
STR. NO.	DETAIL	TOP ELEV.	INV. IN - Underdrain	INV. IN	INV. OUT	REMARKS
MB-1	NDS Basin 12"	391.14	386.14	---	386.04	
MB-2	NDS Basin 12"	389.98	385.48	---	385.38	
MB-3	NDS Basin 12"	388.82	384.32	---	384.22	
MB-4	NDS Basin 12"	387.90	383.40	---	383.30	
MB-7	NDS Basin 12"	384.25	378.50	---	378.40	
MB-9	NDS Basin 12"	383.15	377.40	---	377.30	
MB-10	NDS Basin 12"	382.75	378.25	---	378.15	
MB-11	NDS Basin 12"	383.25	378.75	---	378.65	
MB-12	NDS Basin 12"	383.28	378.78	---	378.68	
MB-13	NDS Basin 12"	382.80	376.55	---	376.45	
MB-16	NDS Basin 12"	383.84	377.09	---	376.99	
MB-17	NDS Basin 12"	383.50	376.75	---	376.65	
MB-18	NDS Basin 12"	382.70	378.20	---	378.10	
MB-19	NDS Basin 12"	381.34	375.09	---	374.99	
MB-20	NDS Basin 12"	380.34	374.59	---	374.49	
MB-21	NDS Basin 12"	379.34	373.59	---	373.49	
MB-22	NDS Basin 12"	378.34	372.59	---	372.49	
MB-23	NDS Basin 12"	377.34	371.59	---	371.49	
MB-24	NDS Basin 12"	377.34	371.59	---	371.49	
MB-25	NDS Basin 12"	378.34	372.59	---	372.49	
MB-26	NDS Basin 12"	379.34	373.59	---	373.49	
MB-27	NDS Basin 12"	380.34	374.59	---	374.49	
MB-28	NDS Basin 12"	381.34	375.09	---	374.99	
MB-29	NDS Basin 12"	383.40	377.65	---	377.55	
MB-30	NDS Basin 12"	384.30	378.55	---	378.45	
MB-31	NDS Basin 12"	391.30	385.05	---	384.95	
MBP-1	NDS Basin 12"	381.00	373.50	---	373.40	



GENERAL NOTES

- REFER TO MARYLAND STATE HIGHWAY ADMINISTRATION FOR MATERIALS AND METHODS OF CONSTRUCTION.
- USE SOLID MASONRY (BRICK OR CONCRETE BLOCK) OR POURED CONCRETE FOR WALLS.
- PAVING OUTSIDE WALLS.
- ADHERE TO ALL SPECIFICATIONS TO THIS TYPE MANHOLE.
- FOR PIPES LARGER THAN 30" PROVIDE STEPS IN CHANNELS OF STRUCTURES. SEE STANDARD DETAIL STANDARD MC-3002E.

DEPARTMENT OF PERMITTING SERVICES
 APPROVED: *[Signature]*
 REVISIONS: *[Table]*
 STANDARD NO. MC-510.01



OWNER
 Woodgen Master Owner, LLC
 4800 Hampton Ln, Suite 800
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 Tel: 301.657.7325

APPLICANT
 NOBE ORC, LLC
 c/o C/S Development
 8245 Boone Blvd, Suite 640
 Tysons Corner, VA 22182
 Tel: 703.520.0828

ARCHITECT
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North Bethesda Market II
 NORTH BETHESDA, MARYLAND

STORMWATER MANAGEMENT DETAILS

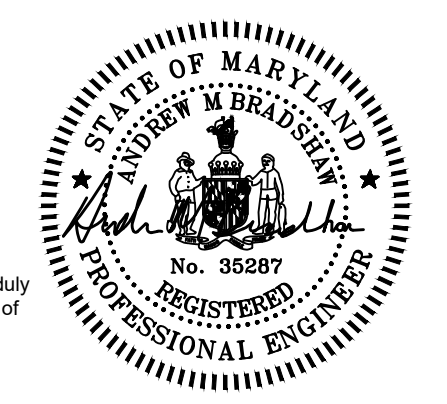
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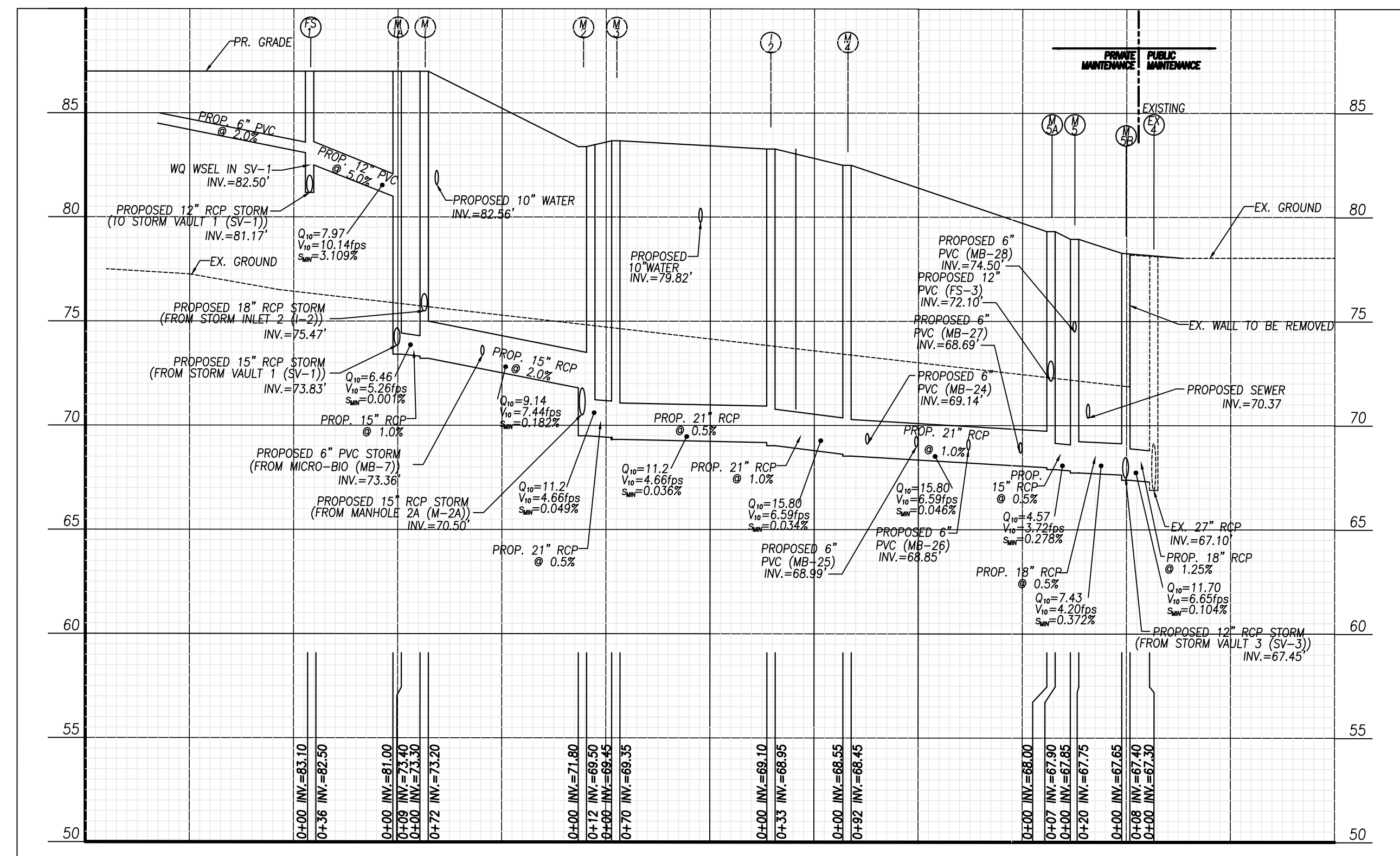
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	Permit Set	10-22-24

KEY PLAN

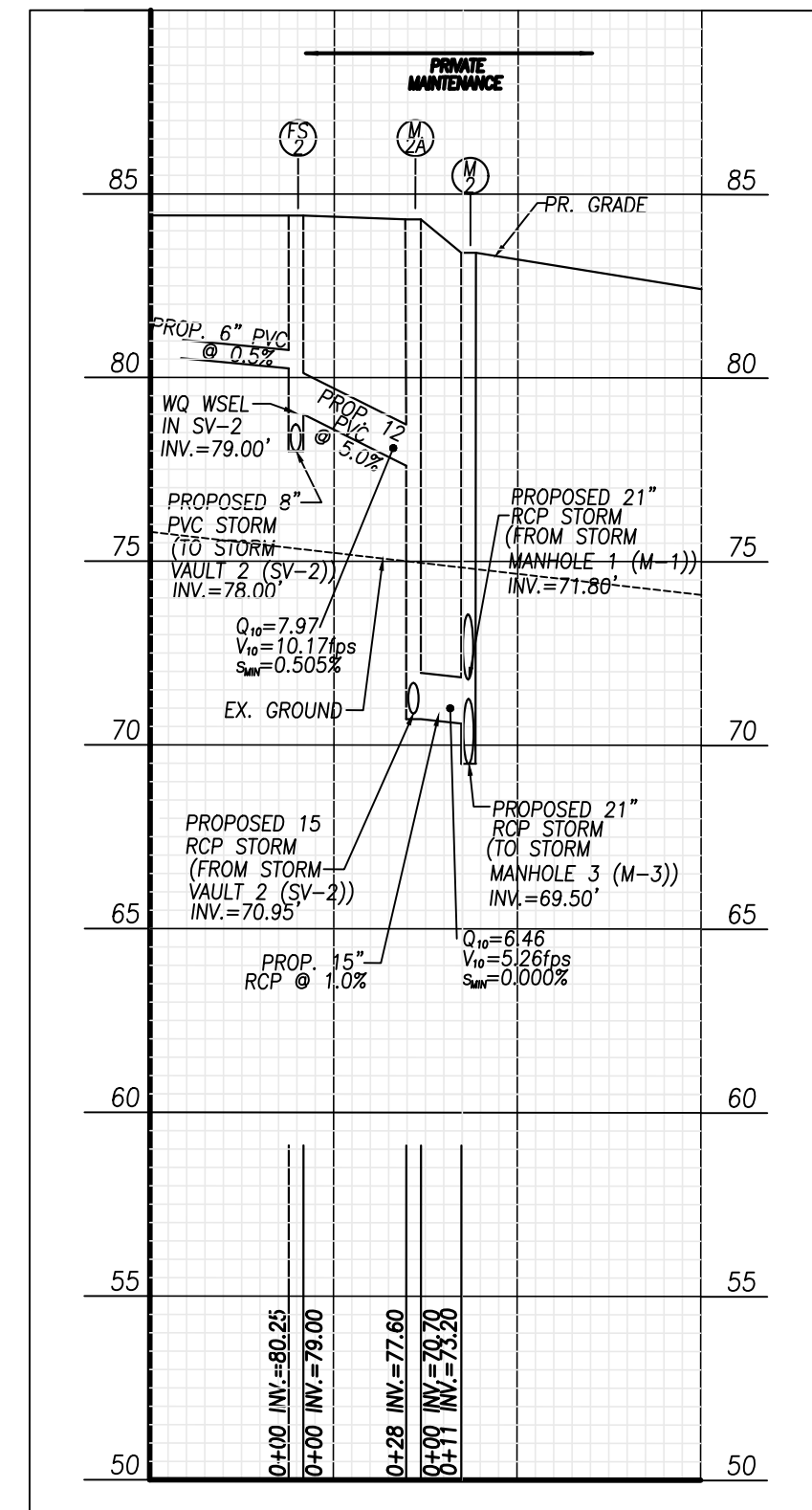
DRAWING STAMP

Professional Certification
 I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No.: 35287
 Expiration Date: 01/01/26

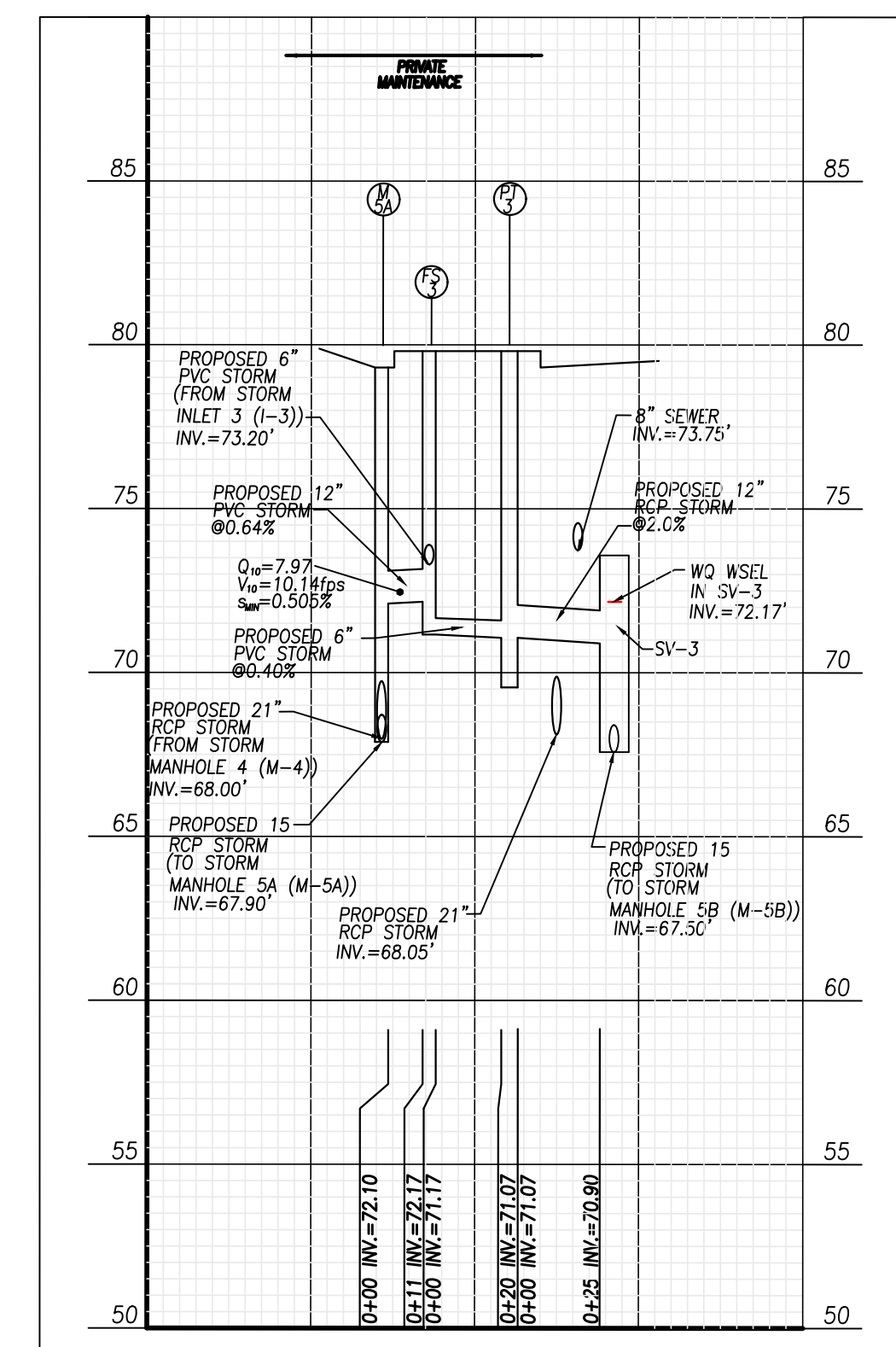




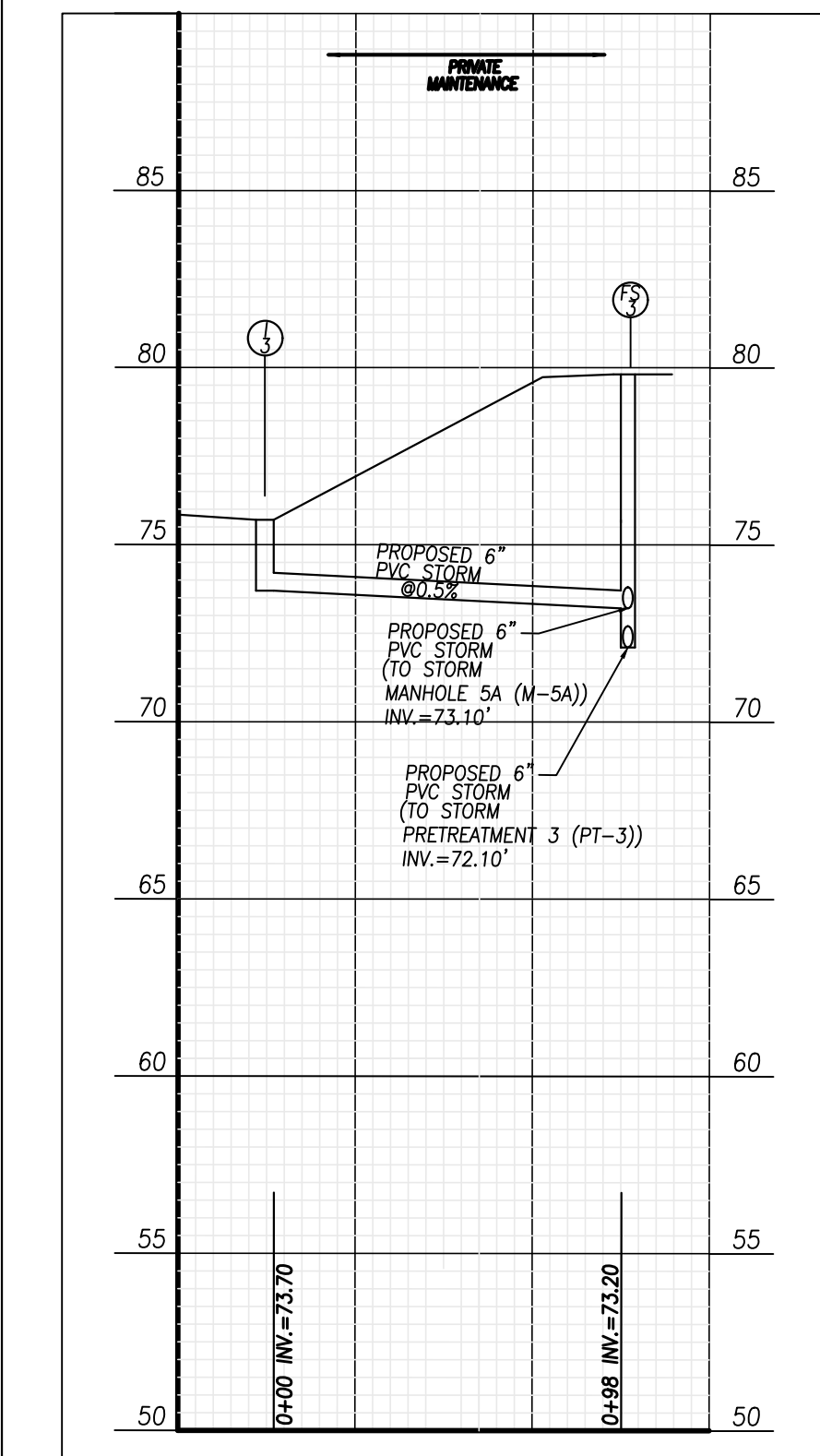
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 1"=5' V



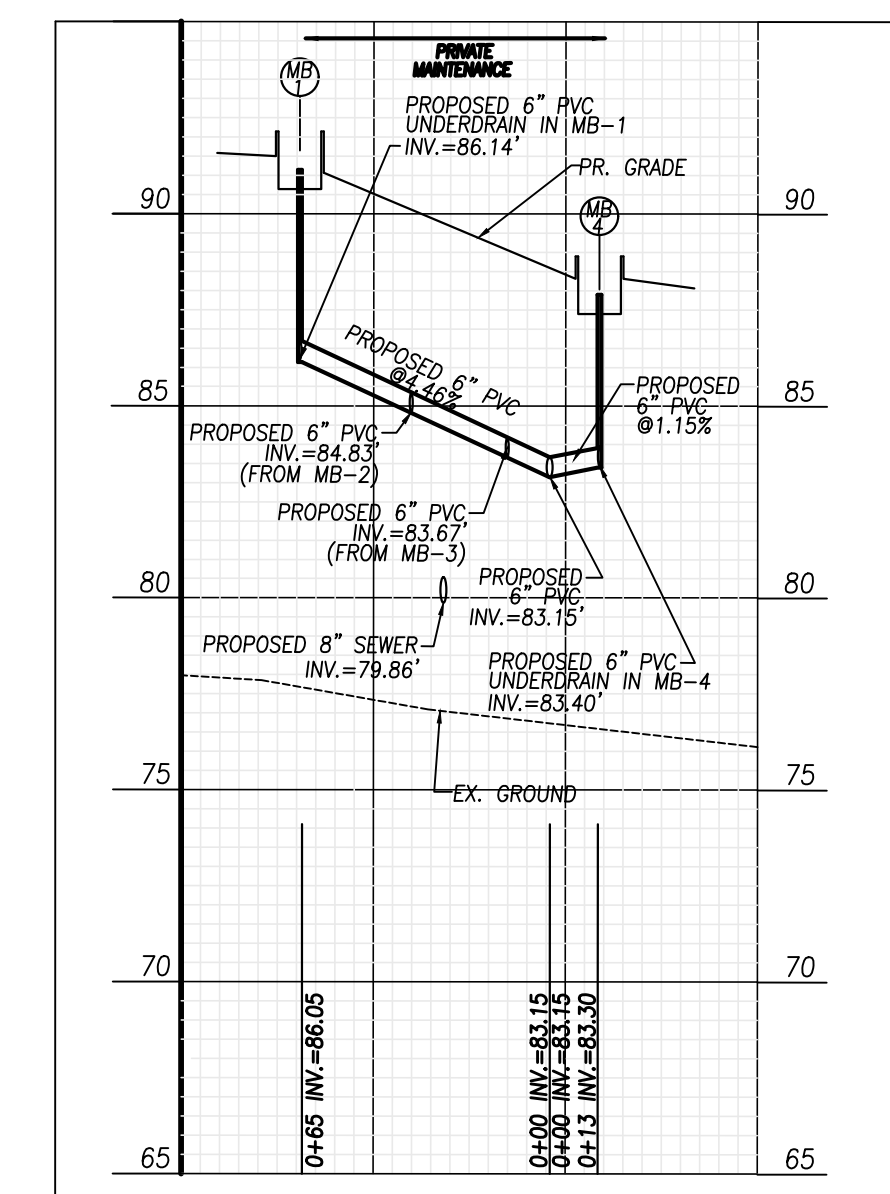
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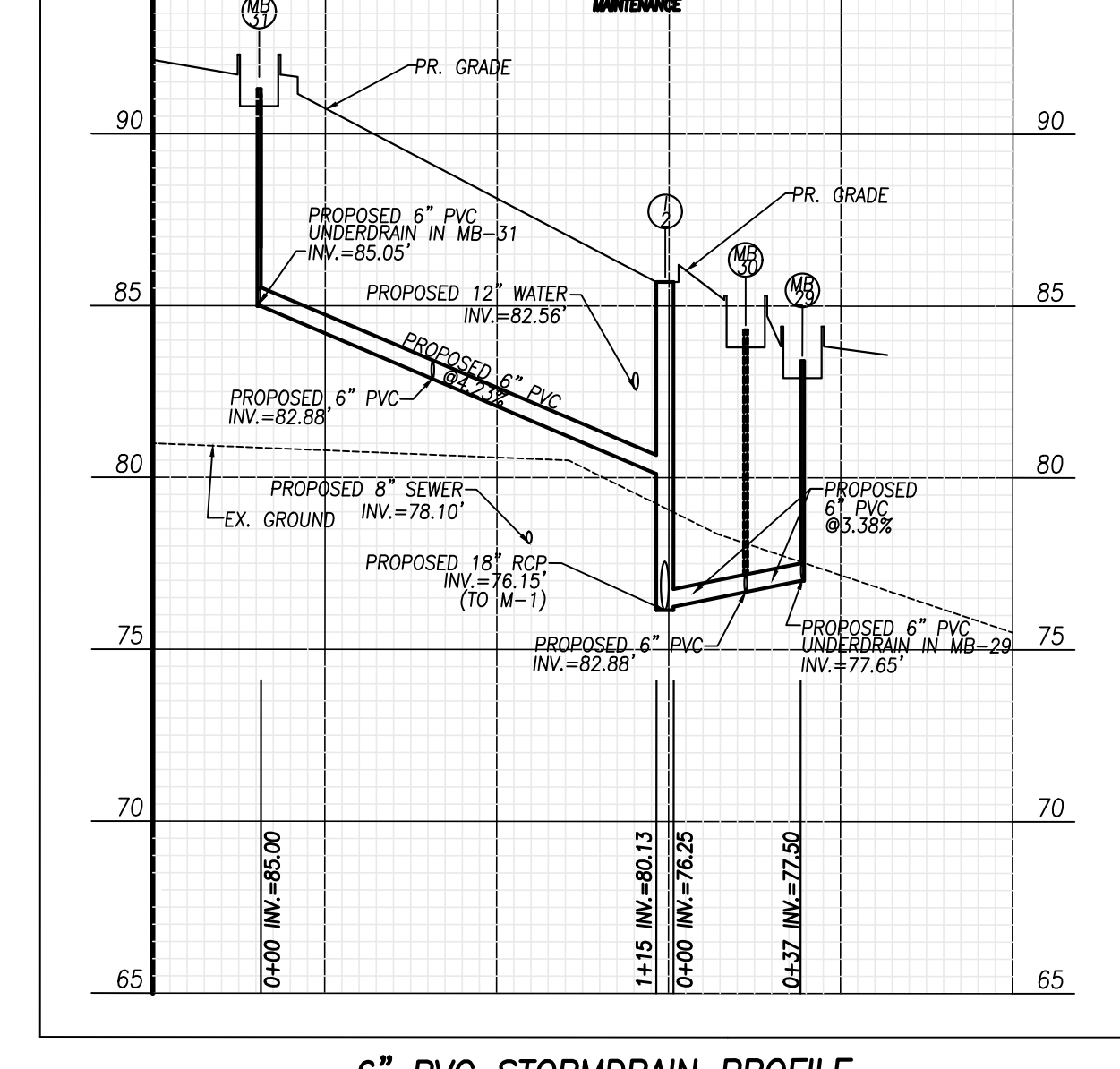
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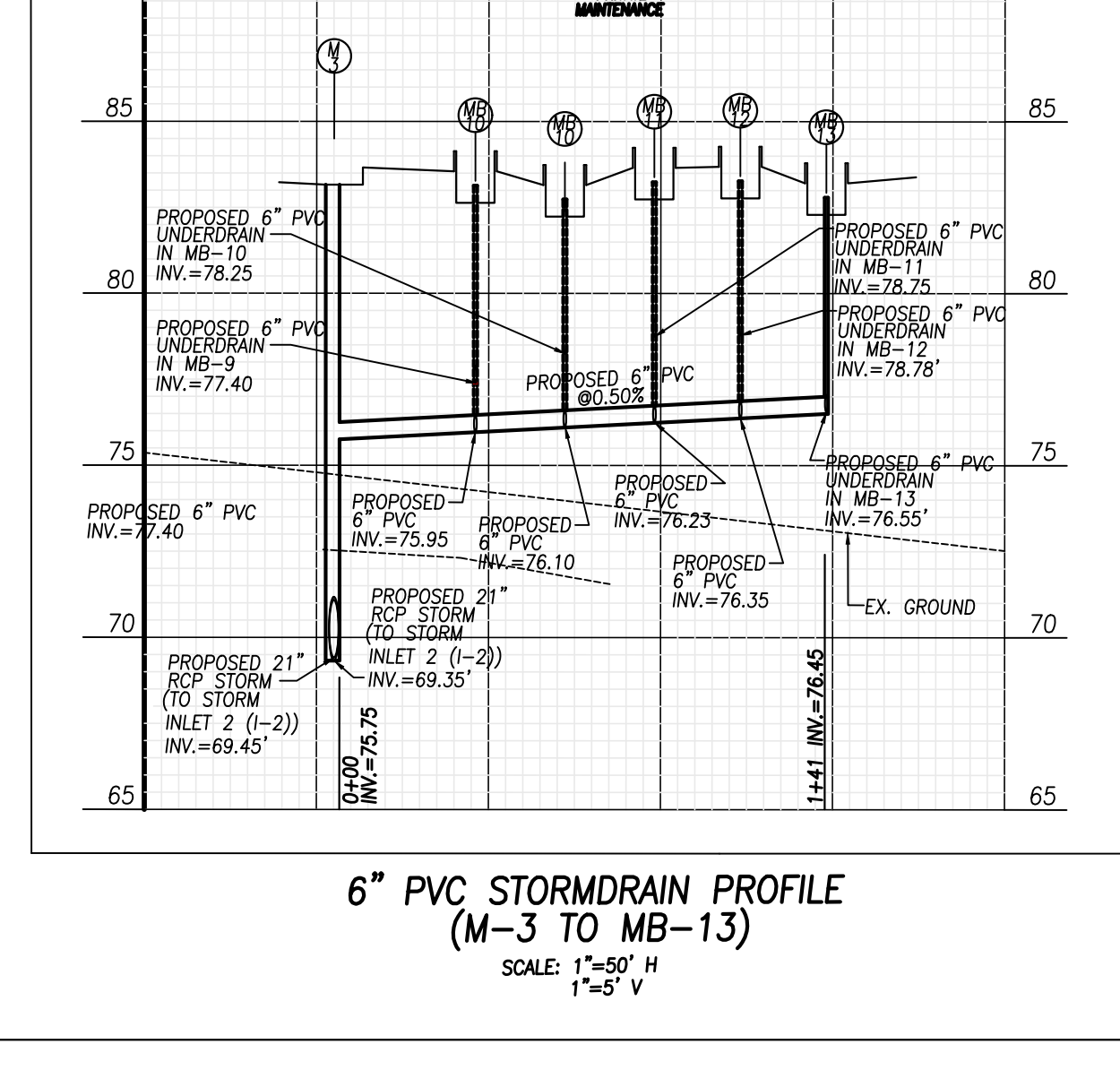
STORMDRAIN PROFILE (I-3 TO FS-3)
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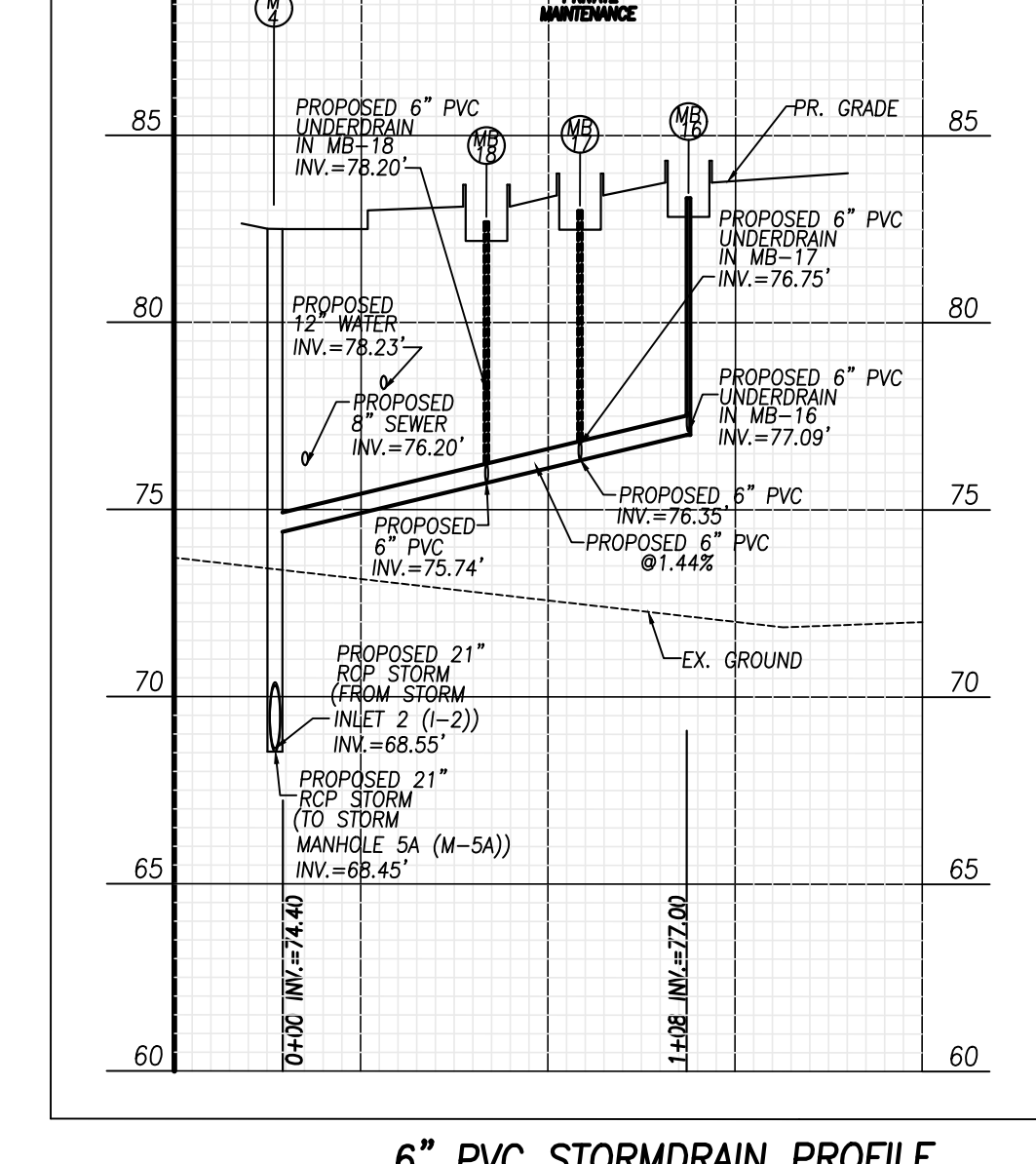
6" PVC STORMDRAIN PROFILE (MB-1 TO MB-4)
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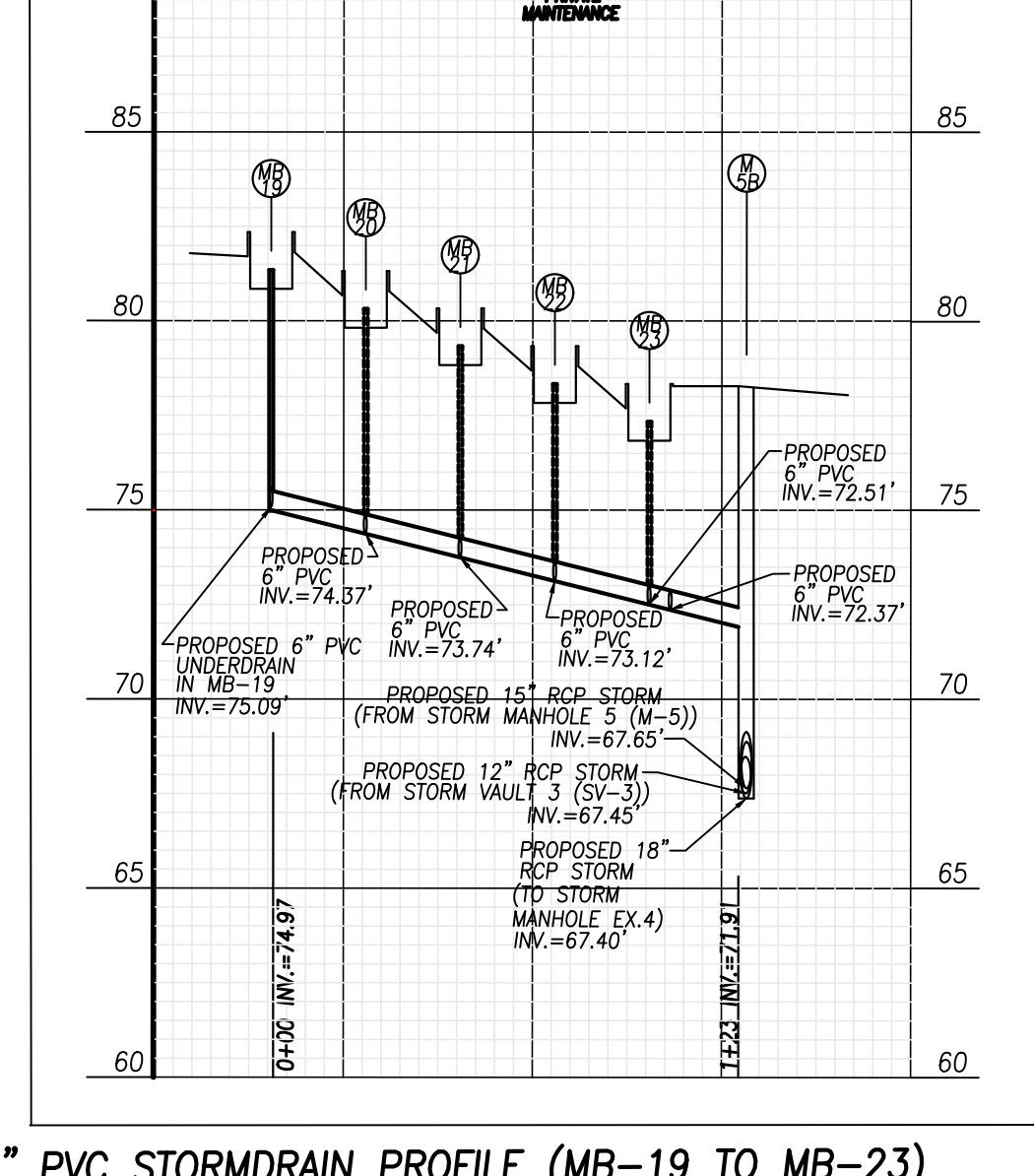
6" PVC STORMDRAIN PROFILE (MB-31 TO I-2 AND I-2 TO MB-29)
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 1"=5' V



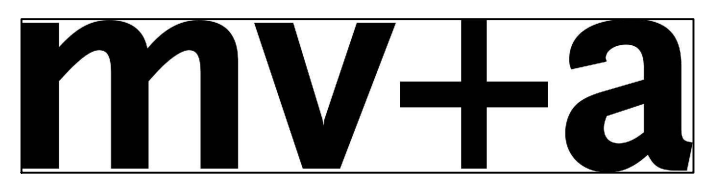
6" PVC STORMDRAIN PROFILE (M-3 TO MB-13)
 SCALE: 1"=50' H
 1"=5' V



6" PVC STORMDRAIN PROFILE (M-4 TO MB-16)
 SCALE: 1"=50' H
 1"=5' V



6" PVC STORMDRAIN PROFILE (MB-19 TO MB-23)
 SCALE: 1"=50' H
 1"=5' V



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North Bethesda Market II

NORTH BETHESDA, MARYLAND

STORMWATER MANAGEMENT PROFILES

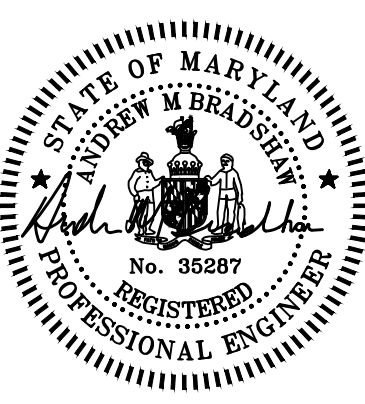
PROJECT NUMBER
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REV.	ISSUE:	DATE
	Permit Set	10-22-24

KEY PLAN

DRAWING STAMP

Professional Certification
 I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No.: 35287
 Expiration Date: 01/07/26



SHEET NUMBER

C-4.07





APPROVED
 Department of Permitting Services
 Permit # **SEDIMENT-297245**
 Date **10/24/2025**
 Stamped By: Mark Etheridge

PLANT LEGEND

SYMBOL	CODE	BOTANICAL / COMMON NAME
DECIDUOUS TREES		
	NYSS	NYSSA SYLVATICA / TUPELO
	QUPH	QUERCUS PHELLOS / WILLOW OAK
DECIDUOUS SHRUBS		
	HAVE	HAMAMELIS VERNALIS 'QUASIMODO' / QUASIMODO OZARK WITCHHAZEL
EVERGREEN SHRUBS		
	ILGL	ILEX X GLABRA 'SHAMROCK' / SHAMROCK INKBERRY
	KALA	KALMIA LATIFOLIA / MOUNTAIN LAUREL
GROUND COVERS		
	EUPU	EUPATORIUM PURPUREUM / JOE PYE WEED
	LOCA	LOBELIA CARDINALIS / CARDINAL FLOWER
	PAVI	PANICUM VIRGATUM / SWITCH GRASS
	RULA	RUDBECKIA LACINIATA / CUTLEAF CONEFLOWER



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NOBE II MULTIFAMILY

7055 GASTRO LANE, ROCKVILLE, MD

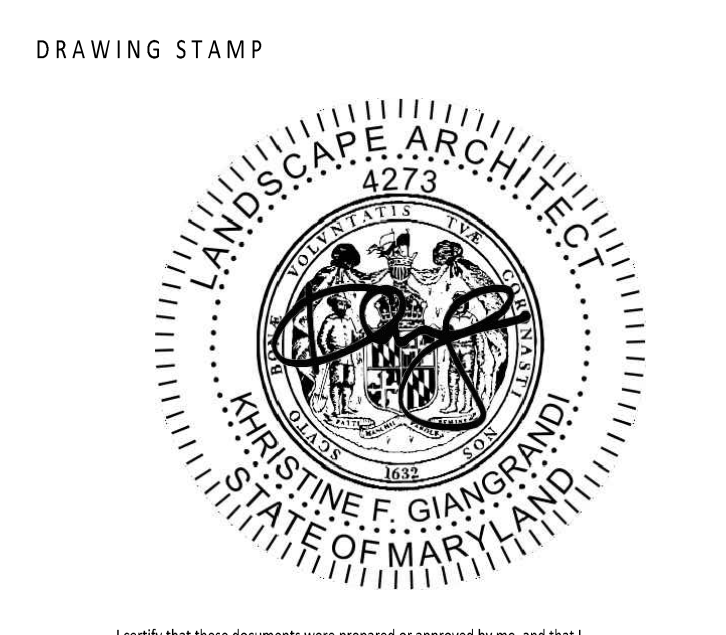
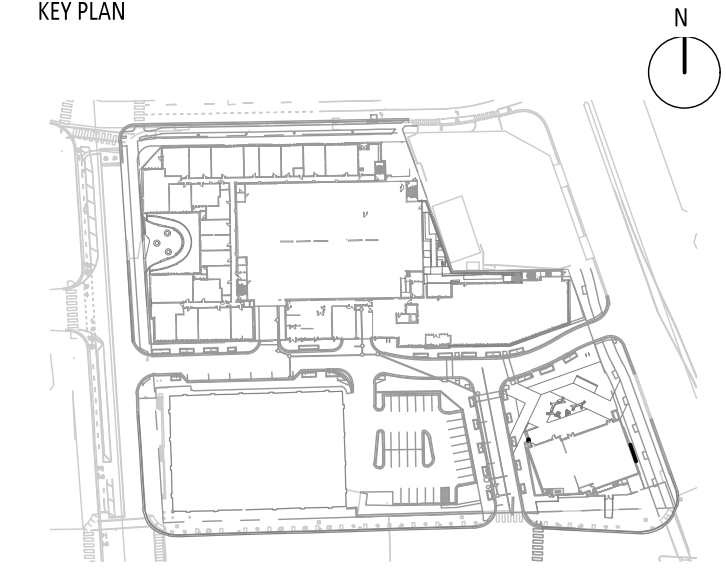
PLANTING PLAN - BIO-RETENTION

PROJECT NUMBER
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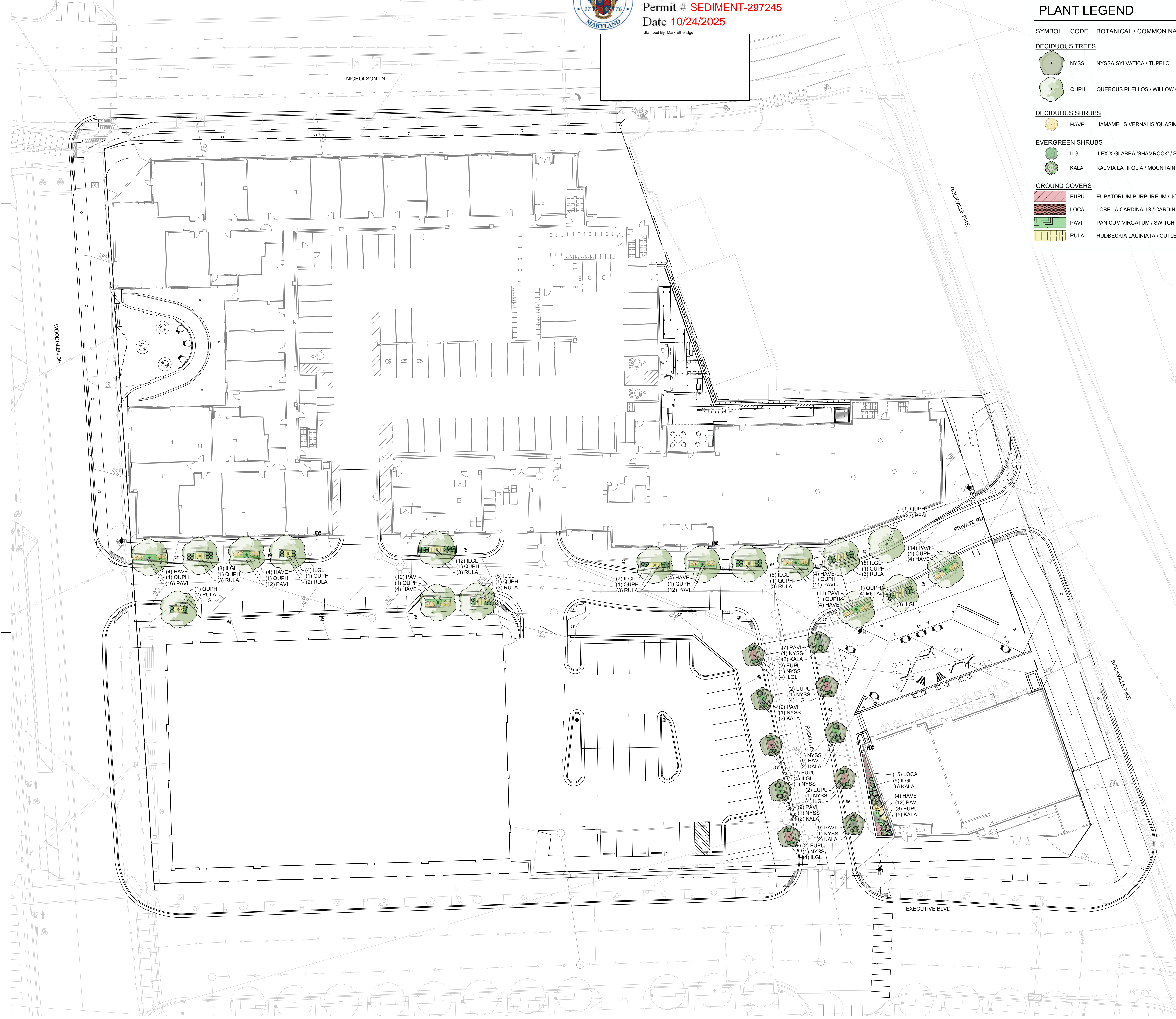
REV.	ISSUE	DATE
Permit Set		10-22-24

DESIGNED BY: KG, TB, ML
 DRAWN BY: KG, TB, ML, JC
 CHECKED BY: KG

SCALE: NORTH
 VERT: N/A
 HORZ: 1"=20'
 0 10' 20' 40'



Earth and those documents were prepared or approved by me, and that I am a duly licensed and active under the laws of the State of Maryland.
 LICENSE NUMBER: 4273 EXPIRATION DATE: 07.26.2026





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NOTE: PRODUCTION WILL NOT COMMENCE UNTIL RECEIPT OF SIGNED APPROVED SHOP DRAWINGS.

PRECAST CONCRETE DESIGN SPECIFICATIONS:

- CONCRETE MINIMUM (28-DAY) COMPRESSIVE STRENGTH: 6,000 PSI
- REINFORCING DEFORMED BAR CONFORMING TO ASTM A615 (FY=60 KSI)
- REINFORCING WELDED WIRE CONFORMING TO ASTM A1064 (FY=80 KSI)
- STRUCTURAL REINFORCING FIBERS CONFORMING TO ASTM C1119
- DESIGN FILL RANGE: 1'-0" MIN - 10'-0" MAX
- DESIGN LIVE LOAD: AASHTO HS-20
- SOIL UNIT WEIGHTS: 120 PCF
- GROUNDWATER TABLE ASSUMED TO BE AT OR BELOW INVERT OF STRUCTURE
- DESIGN CRITERIA IN ACCORDANCE TO ACI-308

WATERTIGHT JOINT NOTES:

- THE JOINT SYSTEM BETWEEN SECTIONS AND CHAMBERS MUST BE WATERTIGHT TO 17.25 PSI HYDROSTATIC PRESSURE AS DEMONSTRATED BY THE COMPLETION OF A LABORATORY WATERTIGHT HYDROSTATIC JOINT SEAL TEST WITNESSED BY AN INDEPENDENT THIRD PARTY. THE TEST SHALL EXPOSE THE JOINT TO A CONTINUOUS HYDROSTATIC PRESSURE OF 15 PSI FOR 48-HOURS AND THEN 17.25 PSI FOR AN ADDITIONAL 15-MINUTES WITH ZERO LEAKAGE. THE HYDROSTATIC TEST REPORT SHALL BE SUBMITTED AND APPROVED PRIOR TO BID.
- AN EXTERIOR LINER IS NOT CONSIDERED AN ACCEPTABLE MEANS OF MAKING THE SYSTEM WATERTIGHT SINCE IT CANNOT MAKE INDIVIDUAL CHAMBERS WATERTIGHT BETWEEN EACH OTHER, ALLOWING SHORT CIRCUITING OF THE FILTER OR CONTROL STRUCTURE TO OCCUR. THUS A LINER IS NOT ALLOWED.
- WATERTIGHTNESS OF THE BETWEEN PRECAST SECTIONS IS THE RESPONSIBILITY OF THE SYSTEM MANUFACTURER.
- A 2-YR MANUFACTURER'S JOINT WATERPROOFING WARRANTY SHALL BE PROVIDED FOR ALL WATERTIGHT SYSTEMS BY THE SYSTEM MANUFACTURER. THE WARRANTY BEGINS ONCE THE WATERTIGHT WORK IS PERFORMED.

CONSTRUCTION NOTES:

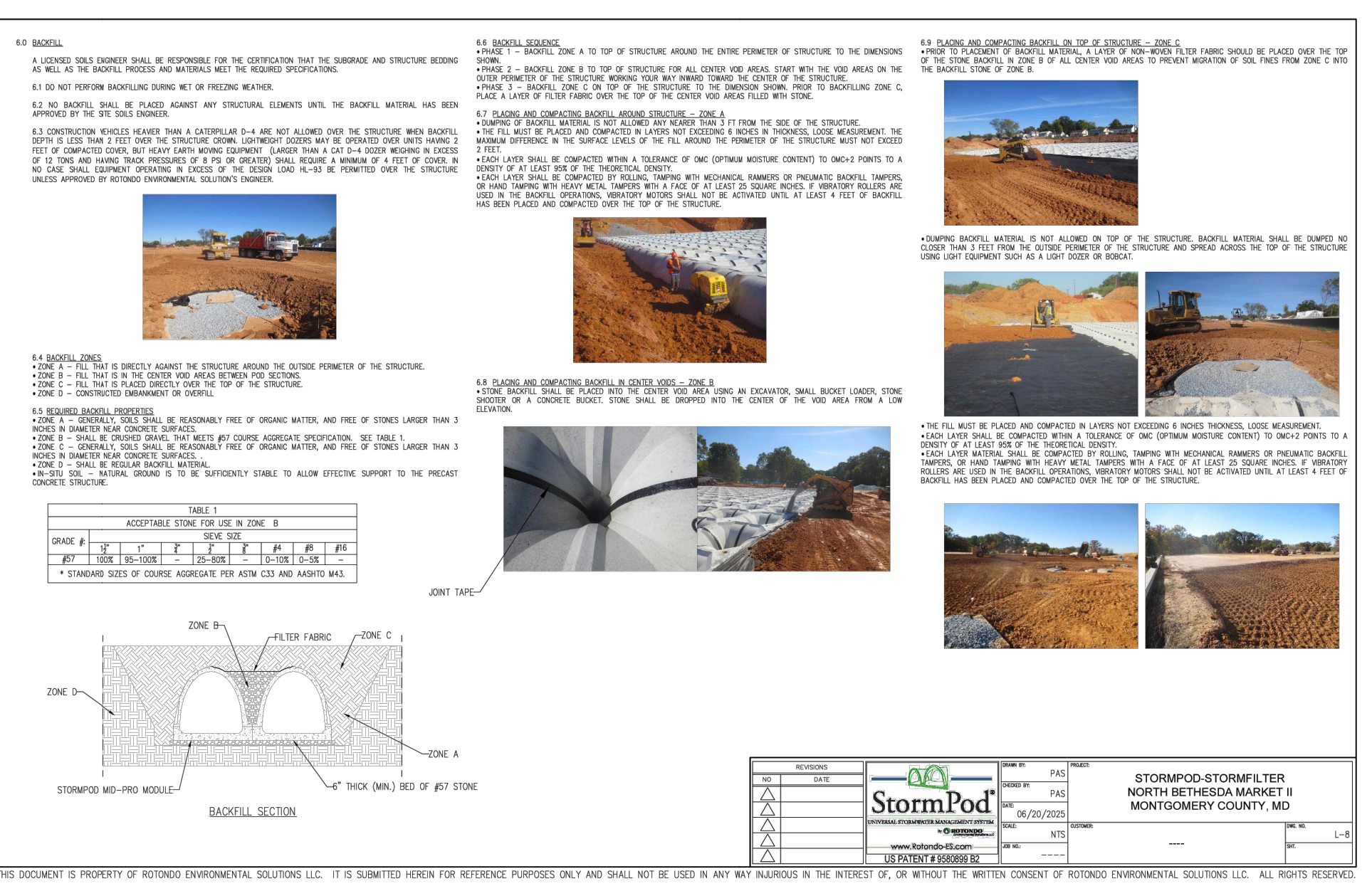
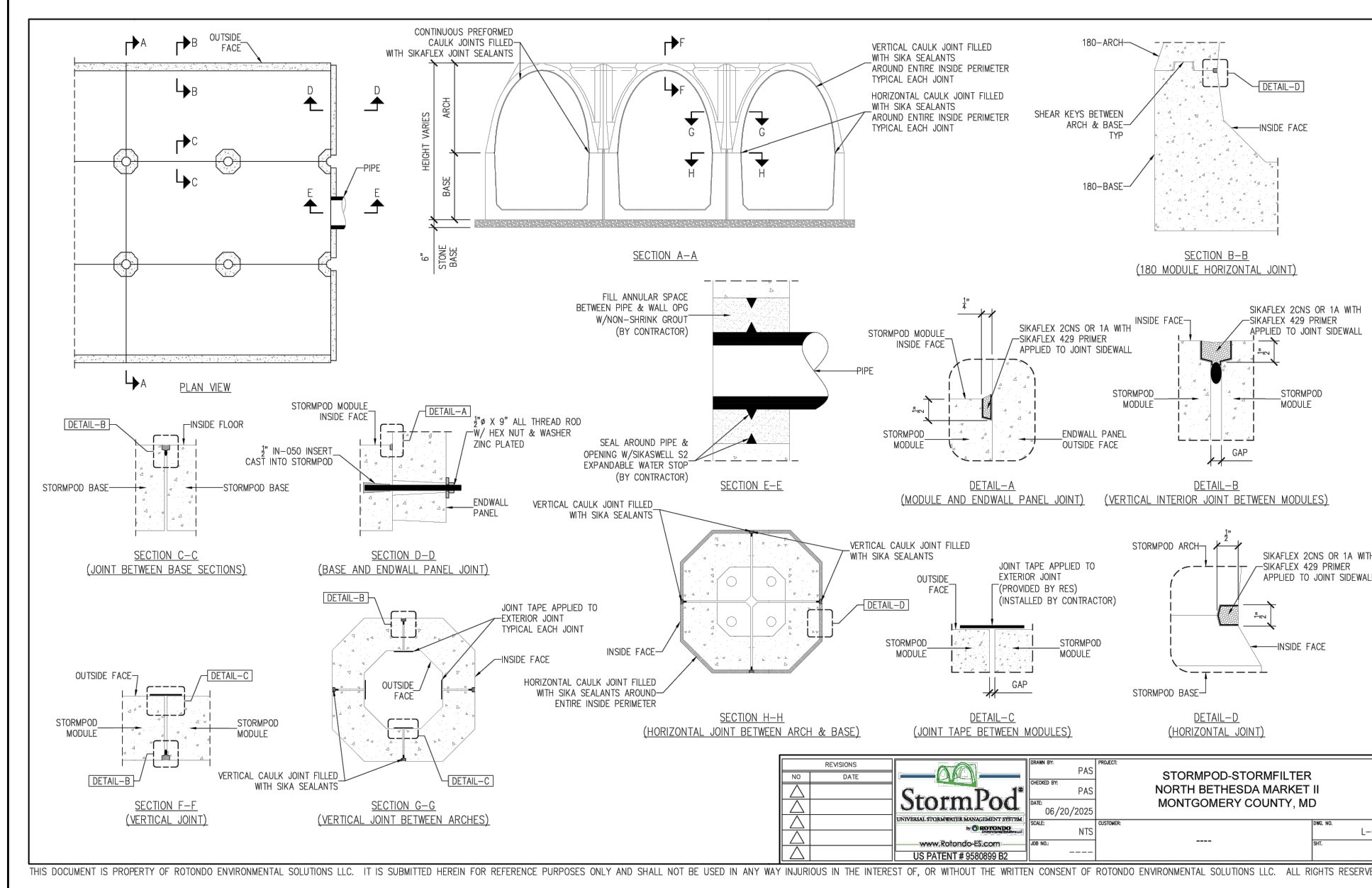
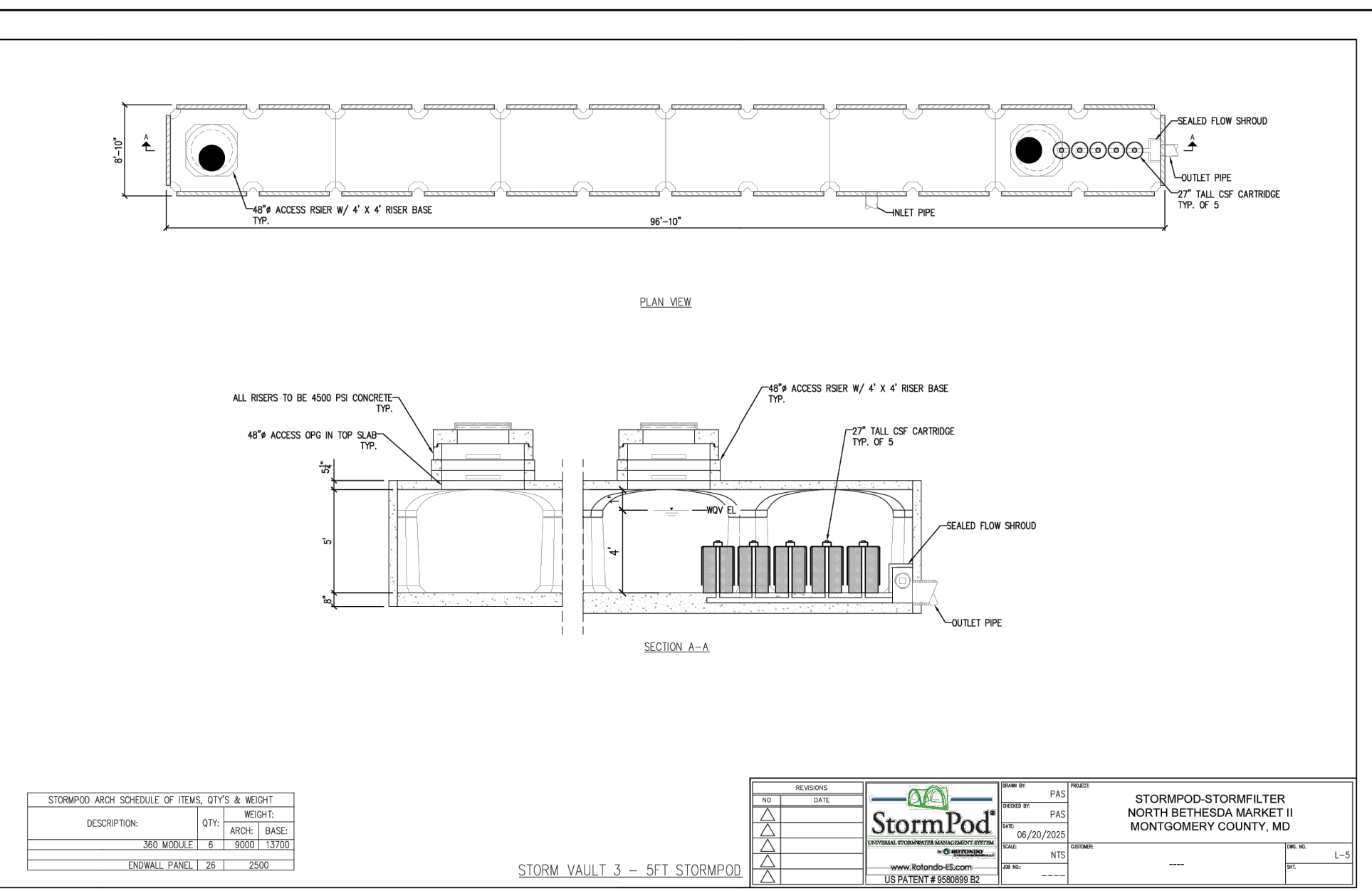
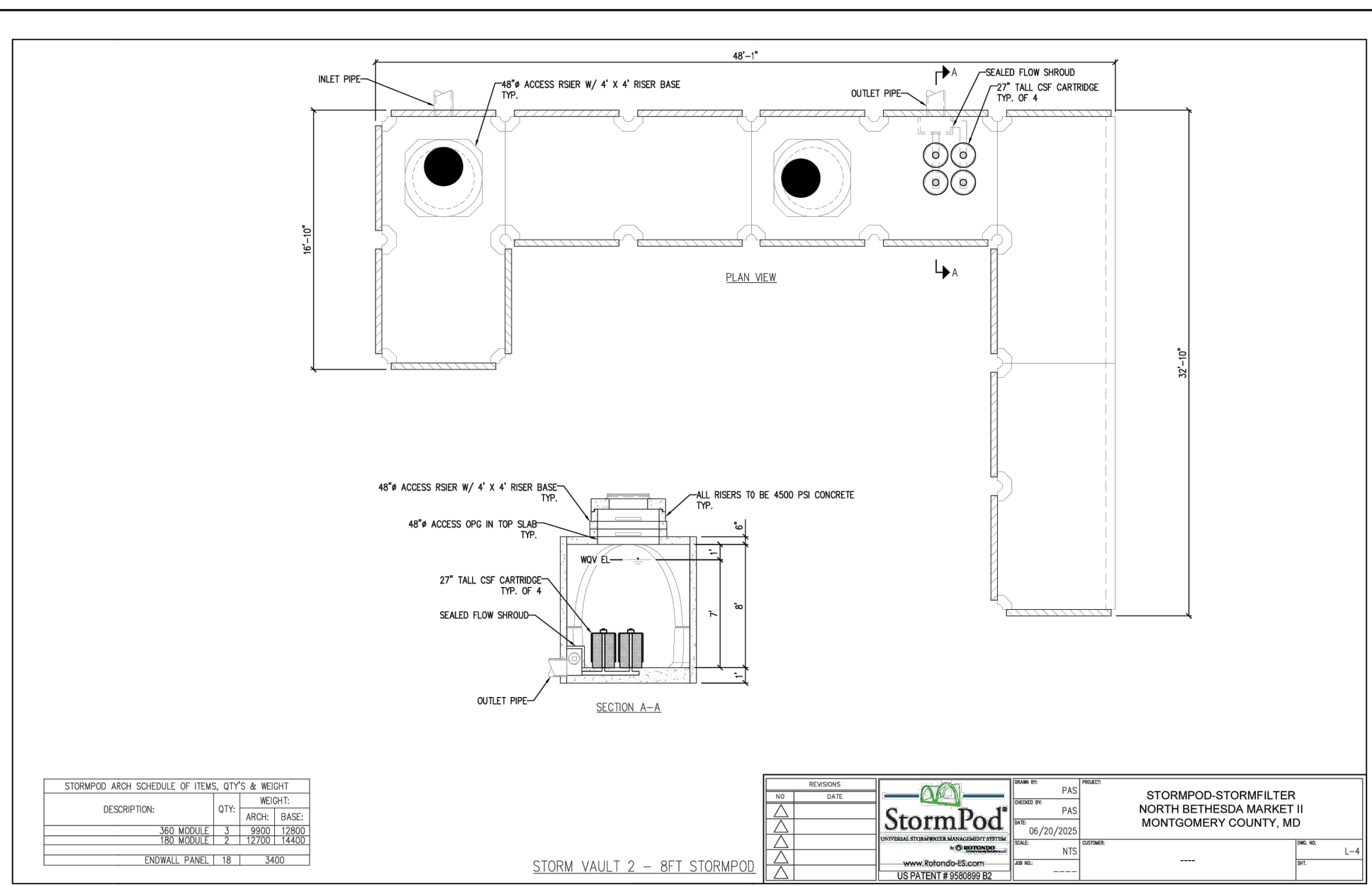
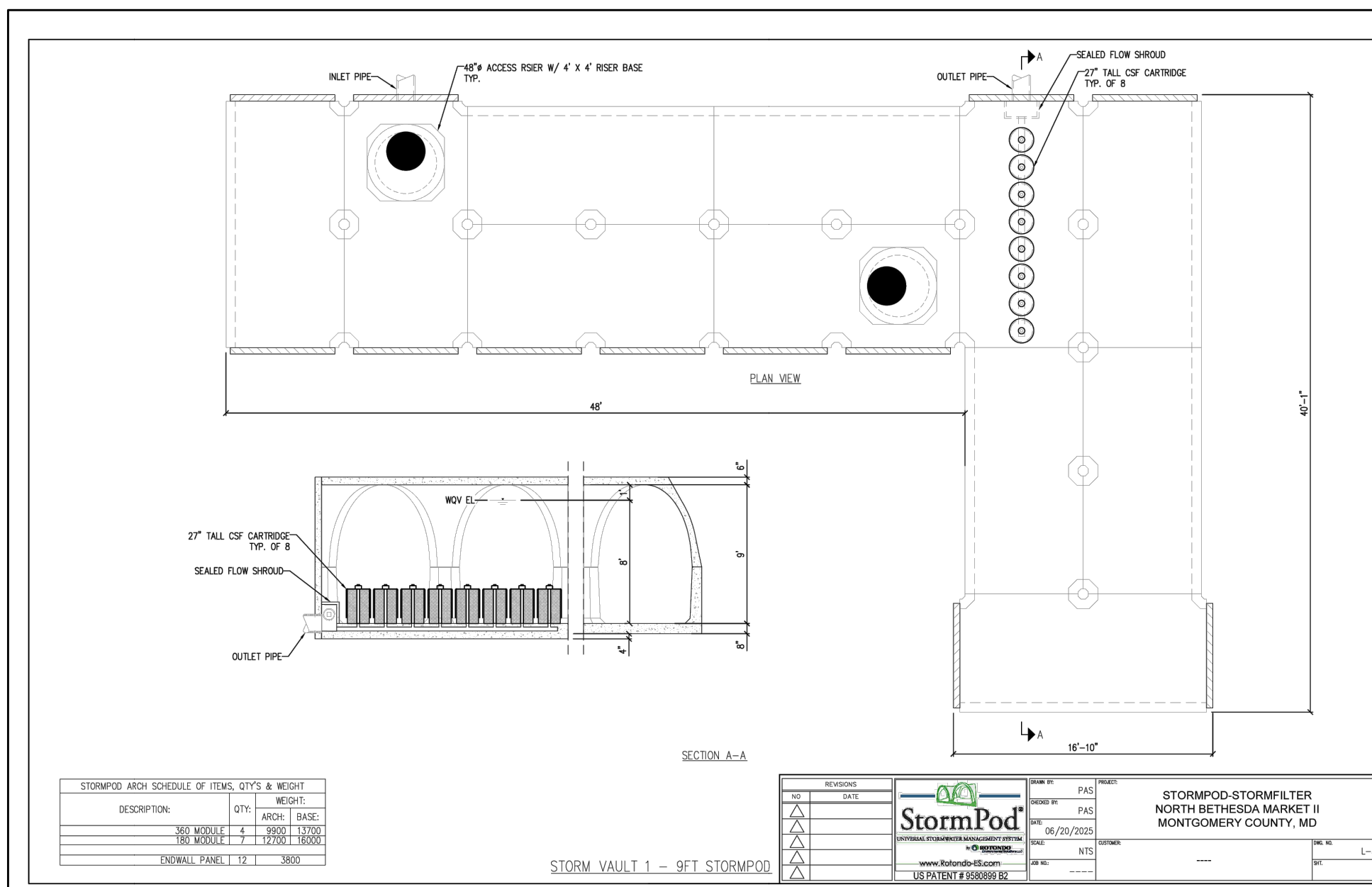
- LATERAL PIPE OPENINGS TO BE CAST INTO THE STRUCTURE AT THE PRECAST FACILITY.
- LATERAL PIPES TO EXTEND BEYOND THE INSIDE WALL AND SEALED WITH GROUT.
- ANNULAR SPACE BETWEEN THE PIPE AND WALL OPENING TO BE SEALED WITH AN EXPANDABLE WATER-STOP AND AN APPROVED NON-SHRINK GROUT.
- MANHOLE ACCESS RISERS AND MANHOLE FRAMES & COVERS TO BE PROVIDED BY THE MANUFACTURER, BUT INSTALLED ON SITE BY THE CONTRACTOR.
- ACCESS RISERS AND MANHOLE COVERS TO BE INSTALLED AND GROUTED ON SITE TO MEET FINISH GRADE BY THE CONTRACTOR.
- LADDERS TO BE PROVIDED AND INSTALLED BY THE MANUFACTURER.
- FABRICATION TO BE IN CONFORMANCE WITH THE LATEST EDITION AND ADDENDA OF THE MSHA STANDARDS AND SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS.
- CONCRETE DESIGN CRITERIA TO MEET ACI 350, ENVIRONMENTAL ENGINEERING STRUCTURES, WITH FREEZING AND THAWING EXPOSURE. CONCRETE SHALL BE A TYPE 1L CEMENT WITH A (28-DAY) COMPRESSIVE STRENGTH OF 5,000 PSI.

STORAGE VOLUME PROVIDED:

SV-1 STORAGE VOLUME PROVIDED:
9,561 ft ³ = Total Storage Volume Provided = V _{flow} + V _{up} + V _{up2} + V _{up3} + V _{up4}

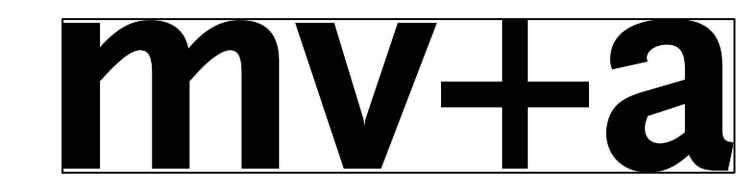
SV-2 STORAGE VOLUME PROVIDED:
3,883 ft ³ = Total Storage Volume Provided = V _{flow} + V _{up} + V _{up2} + V _{up3} + V _{up4}

SV-3 STORAGE VOLUME PROVIDED:
3,416 ft ³ = Total Storage Volume Provided = V _{flow} + V _{up} + V _{up2} + V _{up3} + V _{up4}



NOTES:

1. Shop drawings for SV-1, SV-2, and SV-3 must be prepared, approved by the project engineer, and submitted to DPS plan reviewer for acceptance, utilizing standard precast checklist, prior to fabrication.
2. Structures must be watertight. Leakage test is required prior to backfill. Results of Leakage tests to be provided with the as-built.



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MEP ENGINEER
Scot Engineering, Inc.
509 Germantown

BUILDING CODE AND DESIGN LOADS

- 1. INTERNATIONAL BUILDING CODE (IBC): 2018
2. LOCAL CODE AMENDMENTS: MONTGOMERY COUNTY, MD AMENDMENTS
3. LIVE LOADS
A. SIDEWALKS, VEHICULAR DRIVEWAYS AND YARDS SUBJECT TO TRUCKING: 250 PSF OR 800 LBS. CONCENTRATED
B. HL-93 LOADING
4. SEISMIC LOAD CRITERIA:
A. RISK CATEGORY: II
B. IMPORTANCE FACTOR I: 1.0
C. S_e: 0.135
D. S_u: 0.043
E. S_w: 0.144
F. S_v: 0.069
G. SITE CLASS: D
H. SEISMIC DESIGN CATEGORY: B

GENERAL

- 1. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCES TO ENSURE STABILITY AND SAFETY DURING CONSTRUCTION. THIS INCLUDES BUT IS NOT LIMITED TO, THE ADDITION OF SHEETING, SHORING, TEMPORARY BRACING, GUYS, AND TIEDOWNS. THE CONTRACTOR SHALL PROVIDE SHORING AND BRACING NECESSARY TO PROTECT EXISTING AND ADJACENT STRUCTURES.
2. STRUCTURAL DOCUMENTS SHALL BE USED WITH OTHER CONSTRUCTION DOCUMENTS, INCLUDING ARCHITECTURAL, DEMOLITION, M/E/P, AND CIVIL DRAWINGS, AND THE GEOTECHNICAL REPORT. INFORMATION REQUIRED TO COORDINATE THE STRUCTURAL PORTIONS OF THE WORK INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
A. CIVIL DRAWINGS:
I. PROJECT DATUM AND SITING OF THE BUILDING
II. EXISTING SITE CONDITIONS AND EXISTING UTILITIES
III. SITE PREPARATION AND GRADING
IV. BACK FILL MATERIALS AND BACKFILL REQUIREMENTS
V. NEW UTILITIES
VI. PAVING AND SITE ELEMENTS OUTSIDE OF THE BUILDING ENVELOPE (ALSO REFER TO THE LANDSCAPE DRAWINGS)
VII. DEMOLITION DRAWINGS
B. GEOTECHNICAL REPORT:
I. SUBGRADE PREPARATION REQUIREMENTS
II. MINIMUM FOUNDATION SIZES AND BEARING ELEVATIONS
III. SUBGRADE AND PERIMETER DRAINAGE SYSTEM REQUIREMENTS
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LIMITING CONSTRUCTION LOADS SUCH THAT THESE LOADS DO NOT EXCEED THE DESIGN LIVE LOADS NOTED ABOVE. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AS REQUIRED DURING CONSTRUCTION TO SUPPORT CONSTRUCTION LOADS UNTIL SUCH TIME THAT THE STRUCTURE IS ABLE TO SUPPORT THE DESIGN LIVE LOADS NOTED.
5. SECTIONS AND DETAILS SHOWN ON THE STRUCTURAL DOCUMENTS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS THAT DO NOT HAVE A SPECIFIC SECTION INDICATED.
6. TYPICAL DETAILS APPLY AT ALL APPROPRIATE LOCATIONS AND ARE NOT GENERALLY CUT ON PLANS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TYPICAL DETAIL APPLICATIONS.
7. FOR INCONSISTENCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS, AND CONSTRUCTION DOCUMENTS, THE MOST STRICT REQUIREMENT SHALL APPLY. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.
8. PROVIDE ALL LABOR, MATERIAL, EQUIPMENT AND MISCELLANEOUS ITEMS INCLUDING BUT NOT LIMITED TO CLIPS, INSERTS, TIES, ANCHOR STRAPS, HANGERS, BOLTS, AND OTHER FASTENERS REQUIRED TO COMPLETE THE WORK.

EXISTING CONDITIONS, TEMPORARY SHORING, UNDERPINNING, AND DEMOLITION

- 1. THE DRAWINGS MAY REFLECT INFORMATION PROVIDED BY OTHERS AND/OR EXISTING CONDITIONS THAT HAVE BEEN SURVEYED AND/OR DOCUMENTED TO THE GREATEST POSSIBLE EXTENT BUT NOT VERIFIED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO FULLY COORDINATE THE WORK, INCLUDING, BUT NOT LIMITED TO, THE VERIFICATION OF ALL EXISTING CONDITIONS SHOWN IN THE DRAWINGS AND COORDINATION OF ALL NECESSARY BUILDING TRADES WITH EXISTING CONDITIONS. ANY AND ALL CONDITIONS THAT ARE MISREPRESENTED IN THESE DOCUMENTS OR ANY CONDITIONS THAT ARE NOT SHOWN BUT WARRANT THE ATTENTION OF THE ARCHITECT AND/OR ENGINEER SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT AND/OR ENGINEER.
2. MEANS AND METHODS OF CONSTRUCTION AND TEMPORARY SHORING AND BRACING OF THE EXISTING STRUCTURE(S) AND ADJACENT STRUCTURES ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE ENGINEER MAY INCLUDE PHASING, SEQUENCING, SHORING REQUIREMENTS, ETC. IN THE CONSTRUCTION DOCUMENTS TO ALERT, ASSIST, OR OTHERWISE DICTATE PROCEDURAL REQUIREMENTS THAT MAY BE NECESSARY TO PROPERLY IMPLEMENT THE STRUCTURAL PORTION OF THE WORK OR THAT MAY BE REQUIRED TO ENSURE BUILDING STABILITY; HOWEVER, THE CONTRACTOR SHALL COORDINATE THESE REQUIREMENTS AND SHALL REMAIN COMPLETELY AND SOLELY RESPONSIBLE FOR COMPLETING THE WORK IN A SAFE AND TIMELY MANNER.
3. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS AND METHODS OF DEMOLITION AND THE STABILITY OF THE EXISTING STRUCTURE AND ADJACENT STRUCTURES DURING DEMOLITION OPERATIONS UNTIL THE WORK IS COMPLETE. ALL TEMPORARY SHORING SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER IN THE JURISDICTION OF THE PROJECT AND DEMOLITION WORK SHALL BE COMPLETED BY QUALIFIED AND EXPERIENCED PERSONNEL.
4. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR REPAIRS TO EXISTING STRUCTURAL ELEMENTS THAT MAY BE DAMAGED DURING DEMOLITION OPERATIONS. ANY NECESSARY REPAIR WORK SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER IN THE JURISDICTION OF THE PROJECT AND REPAIRS SHALL BE IMPLEMENTED BY QUALIFIED AND EXPERIENCED PERSONNEL.
5. UNLESS NOTED OTHERWISE, IT HAS BEEN ASSUMED THAT THE EXISTING STRUCTURE(S) ARE IN SERVICEABLE CONDITION, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY AND ALL AREAS OF STRUCTURAL DISTRESS (INCLUDING, BUT NOT LIMITED TO, CRACKS, SPALLING, ETC.) NOT INDICATED IN THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL NOT PROCEED WITH WORK IN SUCH AREAS WITHOUT DIRECTION FROM THE ENGINEER.

FOUNDATIONS

- 1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY UNIVERSA, ENGINEERING SCIENCES, DATED APRIL 4, 2022 AND THE ADDENDUM LETTER DATED DECEMBER 27, 2024. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR THE DESIGN.
2. ASSUMED ALLOWABLE SOIL BEARING VALUE OF 2,000 PSF AND SHALL BE VERIFIED BY A GEOTECHNICAL ENGINEER OR QUALIFIED SOILS TECHNICIAN. REFER TO THE GEOTECHNICAL REPORT FOR EARTHWORK PROCEDURES, COMPACTION, AND ADDITIONAL INFORMATION.
3. ALL FOOTINGS SHALL PROJECT AT LEAST 1'-0" INTO UNDISTURBED NATURAL SOIL OR COMPACTED CONTROLLED FILL HAVING A BEARING VALUE AT LEAST EQUAL TO THAT SPECIFIED ABOVE.
4. BOTTOMS OF ALL EXTERIOR FOOTINGS SHALL BE AT LEAST 2'-6" BELOW FINISHED GRADE OR AS REQUIRED BY LOCAL CODE REQUIREMENTS. FOOTING ELEVATIONS INDICATED ON DRAWINGS HAVE BEEN ESTABLISHED FROM AVAILABLE INFORMATION PROVIDED BY OTHERS AND MAY NOT VIOLATE THE FOREGOING CRITERIA. FOOTING ELEVATIONS SHALL BE LOWERED AS SITE CONDITIONS WARRANT FOR POOR SOIL CONDITIONS OR AS REQUIRED TO FACILITATE SITE UTILITIES OR EXISTING CONDITIONS.
5. UNLESS NOTED OTHERWISE ON THE DRAWINGS, WALL FOOTINGS SHALL BE 12" DEEP AND PROJECT 6" BEYOND EACH FACE OF THE WALL. WALL FOOTINGS SUPPORTING MASONRY WALLS SHALL BE REINFORCED WITH #3@5 CONTINUOUS LONGITUDINAL BOTTOM BARS.
6. ALL DISTURBED EARTH UNDER FOOTINGS SHALL BE REPLACED WITH LEAN CONCRETE.
7. ALL BEARING STRATA SHALL BE ADEQUATELY DRAINED BEFORE FOUNDATION CONCRETE IS PLACED.
8. NO EXCAVATION SHALL BE CLOSER THAN AT A SLOPE OF 2:1 (TWO HORIZONTAL TO ONE VERTICAL) TO AN EXISTING FOOTING OR STRUCTURE UNLESS NOTED OTHERWISE.
9. DO NOT PLACE CONCRETE OVER FROZEN SOIL.
10. UNLESS NOTED OTHERWISE, CENTERLINES OF FOOTINGS SHALL MATCH CENTERLINES OF COLUMNS, PEDESTALS, AND/OR PIERS.
11. MUD MATS SHALL BE PROVIDED TO PROTECT FOUNDATION EXCAVATIONS WHEN NECESSARY.
12. UNLESS NOTED OTHERWISE, THE BUILDING IS NOT DESIGNED FOR HYDROSTATIC PRESSURE. PERMANENT UNDERSLAB AND PERIMETER DRAINAGE SYSTEMS SHALL BE INSTALLED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT, ARCHITECTURAL DRAWINGS, AND PLUMBING DRAWINGS. IN THE ABSENCE OF SPECIFIC REQUIREMENTS, REFER TO IBC SECTION 1805.4. THE CONTRACTOR SHALL COORDINATE THE DRAINAGE SYSTEM(S) WITH THE BUILDING STRUCTURE. DRAINAGE SYSTEMS MAY BE SHOWN ON THE STRUCTURAL DRAWINGS FOR INFORMATIONAL PURPOSES ONLY.
13. FOUNDATION / RETAINING WALLS HAVE BEEN DESIGNED USING THE PROPERTIES LISTED BELOW AS INDICATED IN THE GEOTECHNICAL REPORT. ALL FOUNDATION WALLS ARE TO BE BACKFILLED WITH SLECT FILL AS INDICATED IN THE GEOTECHNICAL REPORT.
A. SOIL DENSITY, SATURATED UNIT WEIGHT: 135 PCF
B. COEFFICIENT OF FRICTION (CONCRETE TO SOIL): 0.35
C. AT-REST HORIZONTAL COEFFICIENT (K₀): 0.5
D. AT-REST PRESSURE: 67.5 PSF/FT
E. ACTIVE PRESSURE HORIZONTAL COEFFICIENT (K_a): 0.33
F. ACTIVE PRESSURE: 45 PSF/FT
G. PASSIVE PRESSURE HORIZONTAL COEFFICIENT (K_p): 3.0
H. PASSIVE PRESSURE: 405 PSF/FT

TEMPORARY SUPPORT OF EXCAVATION

- 1. EXCAVATION SUPPORT SYSTEMS (SHEETING AND SHORING OR OTHER MEANS) SHALL BE DESIGNED AND INSTALLED BY A SPECIALTY CONTRACTOR WITH A MINIMUM OF 5 YEARS OF EXPERIENCE IN THE DESIGN AND INSTALLATION OF EARTH RETENTION SYSTEMS. THE DESIGN OF EARTH RETENTION SYSTEMS SHALL BE PROVIDED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MARYLAND.
2. SIGNED AND SEALED SHOP DRAWINGS AND STRUCTURAL CALCULATIONS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER FOR REVIEW.
3. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFICATION ALL EXISTING CONDITIONS (INCLUDING UTILITIES AND BUILDING SYSTEMS) AND COORDINATION OF EXCAVATION SUPPORT SYSTEMS WITH THESE CONDITIONS.
4. SHEETING AND SHORING OR OTHER EARTH RETENTION SYSTEMS MAY BE SHOWN IN THE STRUCTURAL DRAWINGS FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR COORDINATION OF THESE SYSTEMS WITH THE PERMANENT STRUCTURE AS SHOWN IN THE STRUCTURAL DRAWINGS.
5. A STANDARD PENETRATION TEST SHALL BE MADE AT THE BOTTOM OF EVERY CAISSON BY A SOILS CONSULTANT TO VERIFY THE SOIL BEARING CAPACITY.

CONCRETE

- 1. ALL CONCRETE CONSTRUCTION INCLUDING DETAILING, FABRICATION, PLACEMENT OF REINFORCING, MIXING, HANDLING, PLACING, FINISHING, AND CURING SHALL CONFORM TO ACI-301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE", ACI-315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", AND ACI-318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY".
2. ALL CONCRETE SHALL CONFORM TO ASTM C34. DO NOT PROVIDE AIR ENTRAINMENT FOR CONCRETE THAT WILL RECEIVE A SMOOTH, DENSE, HARD-TROWELED FINISH. PROVIDE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AND WATER/CEMENT RATIOS:

Table with 5 columns: APPLICATION, ACI CLASSIFICATION* (ACI-318 CHAPTER 19), FC, MAXIMUM W/C, TARGET AIR CONTENT. Rows include FOOTINGS and MICRO-BIORETENTION STRUCTURES (WALLS AND SLABS).

***REFER TO ACI-318 TABLE 19.3.2.1 FOR ADDITIONAL REQUIREMENTS AND INFORMATION**

- 3. THE CONTRACTOR SHALL PROVIDE CONCRETE MIX DESIGN DATA CONFORMING TO CHAPTER 19 OF ACI 318 AND ACI 350, ENVIRONMENTAL ENGINEERING CONCERTE STRUCTURES FOR EACH TYPE AND STRENGTH OF CONCRETE SPECIFIED. MIX DESIGN DATA SHALL INCLUDE CONCRETE STRENGTH, SLUMP, AIR ENTRAINMENT, PROPOSED AGGREGATES, ADMIXTURES, POZZOLANS, AND LABORATORY TEST DATA. ALL CONCRETE SHALL ALSO BE IN CONFORMANCE WITH THE LATEST MSHA STANDARDS AND SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS.
4. MATERIALS USED IN CONCRETE MIXES SHALL CONFORM TO THE FOLLOWING STANDARDS:
A. CEMENT TYPE II, CONFORMING TO ASTM C-595 OR TYPE IIA CEMENT.
B. FLY ASH CLASS C & F CONFORMING TO ASTM C618. FLY ASH SHALL BE LIMITED TO A MAXIMUM OF 20% OF TOTAL CEMENTITIOUS MATERIALS BY WEIGHT AND SHALL NOT BE USED IN COLD WEATHER AND EXTERIOR APPLICATIONS.
C. GROUND GRANULATED BLAST-FURNACE SLAG GRADE 100 & 120 CONFORMING TO ASTM C989. SLAG SHALL BE LIMITED TO A MAXIMUM OF 50% OF TOTAL CEMENTITIOUS MATERIALS BY WEIGHT IN TYPICAL APPLICATIONS AND 25% IN COLD WEATHER AND EXTERIOR APPLICATIONS.
D. AIR-ENTRAINED ADMIXTURES CONFORMING TO ASTM C260
E. ADDITIONAL ADMIXTURES SHALL CONFORM TO ASTM C494 AND ASTM C1017
5. CONCRETE AGGREGATES SHALL CONFORM TO THE FOLLOWING STANDARDS:
A. NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C33
B. MAXIMUM AGGREGATE SIZE FOR CONCRETE: 1 IN.
C. MAXIMUM AGGREGATE SIZE FOR PEA-GRAVEL CONCRETE: 3/8 IN.
6. PROPORTION AND DESIGN MIXES TO RESULT IN A CONCRETE SLUMP OF 3 1/2 IN. ± 1 IN. AT THE POINT OF PLACEMENT. CONCRETE CONTAINING HIGH-RANGE WATER REDUCING ADMIXTURES (HRWR) SHALL HAVE A SLUMP OF 4 IN. TO 8 IN.
7. PROVIDE VERTICAL CONTROL JOINTS AT 25 FT. O.C. AT ALL EXPOSED CP WALLS. LOCATE JOINTS BESIDE PIERS INTEGRAL WITH WALLS, NEAR CORNERS, AND IN CONCEALED LOCATIONS WHERE POSSIBLE. COORDINATE JOINT LOCATIONS WITH ARCHITECTURAL DRAWINGS.
8. THE OWNER SHALL RETAIN THE SERVICES OF A QUALIFIED TESTING AGENCY TO PROVIDE TESTING OF CONCRETE TO INCLUDE COMPRESSIVE STRENGTH, TEMPERATURE, SLUMP, AND AIR ENTRAINMENT. SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 150 CUBIC YARDS OF CONCRETE PLACED, NOR LESS THAN ONCE FOR EACH 5,000 SQ. FT. OF SURFACE AREA OF SLABS OR WALLS. EACH SAMPLE SHALL INCLUDE THE FOLLOWING:
A. SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN AS FOLLOWS:
I. WALLS: UNLESS OTHERWISE INDICATED OBTAIN AT LEAST ONE COMPOSITE SAMPLE FOR EACH 150 CU YD OR 5000 SQ. FT OF SURFACE AREA.
II. FOUNDATIONS: OBTAIN AT LEAST ONE COMPOSITE SAMPLE FOR EACH 150 CU YD.
III. FRAMED SLABS: OBTAIN AT LEAST ONE COMPOSITE SAMPLE FOR EACH 100 CU YD OR 5000 SQ. FT OF SURFACE AREA.
IV. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
B. COMPRESSION TEST SPECIMENS PER ASTM C 311C 31M SHALL BE AS FOLLOWS:
I. WALLS: UNLESS OTHERWISE INDICATED CAST AND LABORATORY CURE THREE SETS OF THREE STANDARD 4'X8" CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
II. FRAMED SLABS: CAST AND LABORATORY CURE THREE SETS OF THREE STANDARD 4'X8" CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE. CAST AND FIELD CURE ONE SET OF TWO STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
III. (3) 4'X8" CYLINDERS MAY BE USED IN LIEU OF (2) 6'X12" CYLINDERS WITH APPROVAL OF THE ENGINEER OF RECORD.
C. COMPRESSION STRENGTH TESTS PER ASTM C 39C 39M SHALL BE AS FOLLOWS:
I. WALLS: UNLESS OTHERWISE INDICATED) TEST ONE SET OF LABORATORY-CURED SPECIMENS AT 7 DAYS AND ONE SET OF LABORATORY-CURED SPECIMENS AT 28 DAYS. HOLD ONE SET OF LABORATORY-CURED SPECIMENS IN RESERVE.
II. FRAMED SLABS: TEST ONE SET OF LABORATORY-CURED SPECIMENS AT 7 DAYS, ONE SET OF LABORATORY-CURED SPECIMENS AT 28 DAYS, AND ONE SET OF FIELD-CURED SPECIMENS AT 28 DAYS. HOLD ONE SET OF LABORATORY-CURED SPECIMENS IN RESERVE.
9. PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318 AS REQUIRED, SUBJECT TO THE FOLLOWING:
A. PROVIDE A SHOP DRAWING INDICATING LOCATIONS OF PROPOSED CONSTRUCTION JOINTS. LOCATIONS OF CONSTRUCTION JOINTS ARE SUBJECT TO APPROVAL OF THE ARCHITECT. CONSTRUCTION JOINT LOCATIONS SHALL BE INDICATED ON THE REINFORCING STEEL SHOP DRAWINGS.
B. CONSTRUCT JOINTS TRUE TO LINE WITH FACES PERPENDICULAR TO THE SURFACE PLANE OF CONCRETE, AND INSTALL JOINTS SO THE STRENGTH AND APPEARANCE OF CONCRETE ARE NOT IMPAIRED. PLACE JOINTS PERPENDICULAR TO THE MAIN REINFORCEMENT, AND CONTINUE REINFORCEMENT ACROSS CONSTRUCTION JOINTS UNLESS OTHERWISE INDICATED.
C. FORM KEYED JOINTS AS INDICATED. EMBED KEYS AT LEAST 1-1/2 INCHES (38 MM) INTO CONCRETE.
D. LOCATE JOINTS FOR BEAMS, SLABS, JOISTS, AND GIRDERS IN THE MIDDLE THIRD OF SPANS. OFFSET JOINTS IN GIRDERS A MINIMUM DISTANCE OF TWICE THE BEAM WIDTH FROM A BEAM-GIRDER INTERSECTION.
E. LOCATE HORIZONTAL JOINTS IN WALLS AND COLUMNS AT UNDERSIDE OF FLOORS, SLABS, BEAMS, AND GIRDERS AND AT THE TOP OF FOOTINGS OR FLOOR SLABS.
F. PROVIDE CONTINUOUS WATERSTOPS AT ALL CONSTRUCTION JOINTS EXPOSED TO SOIL OR WATER. REFER TO THE ARCHITECTURAL DRAWINGS AND/OR SPECIFICATIONS FOR ADDITIONAL INFORMATION

REINFORCEMENT STEEL

- 1. CONCRETE REINFORCING SHALL CONFORM TO THE FOLLOWING STANDARDS:
A. DEFORMED BARS: ASTM A615, GRADE 60
B. DEFORMED BARS (EPOXY-COATED): ASTM A775 & A615, GRADE 60
C. PLAIN STEEL WELDED WIRE REINFORCEMENT: ASTM A1064
D. EPOXY-COATED WELDED WIRE REINFORCEMENT: ASTM A884
2. FABRICATE AND PROVIDE STANDARD SUPPORTING ACCESSORIES IN ACCORDANCE WITH THE ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, ACI 315, AND CRSI'S MANUAL OF STANDARD PRACTICE. PROVIDE EITHER CRSI CLASS 1 BAR SUPPORTS OR PRECAST CONCRETE BAR SUPPORTS FOR REINFORCING IN FOUNDATIONS AND SLABS ON GRADE.
3. UNLESS NOTED OTHERWISE, REINFORCING SHALL BE CONTINUOUS WITH CLASS B LAP SPLICES, HOOKS SHALL BE STANDARD HOOKS, AND WALL INTERSECTIONS SHALL HAVE CORNER BARS LAP WELDED WIRE MESH SUCH THAT THE DISPLAY OF THE OUTERMOST CROSS-WIRES OF EACH ADJOINING SHEET IS NOT LESS THAN THE SPACING OF THE CROSS-WIRES PLUS 2 IN. REFER TO THE TYPICAL DETAILS FOR ADDITIONAL DETAILING REQUIREMENTS.
4. ALL REINFORCING FOR STORM WATER MANAGEMENT STRUCTURES AND MICRO-BIORETENTION STRUCTURES SHALL BE EPOXY COATED.
5. CONCRETE PROTECTION FOR REINFORCEMENT:
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 IN.
B. CONCRETE EXPOSED TO EARTH OR WEATHER NO. 6 AND LARGER: 2 IN.
C. CONCRETE EXPOSED TO EARTH OR WEATHER NO. 5 AND SMALLER: 1-1/2 IN.

SHOP DRAWING SUBMITTALS AND DELEGATED DESIGN ITEMS

- 1. THE CONTRACTOR SHALL RETAIN THE NECESSARY DESIGN PROFESSIONALS LICENSED IN THE JURISDICTION OF THE PROJECT TO FULLY DESIGN AND DETAIL THE FOLLOWING DELEGATED DESIGN ITEMS. DELEGATED DESIGN SUBMITTALS SHALL INCLUDE SIGNED AND SEALED DRAWINGS AND CALCULATIONS:
A. TEMPORARY EXCAVATION SUPPORT SYSTEMS
B. TEMPORARY SHORING AND BRACING SYSTEMS
C. CONCRETE MIX DESIGNS
2. THE CONTRACTOR SHALL REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO THE DESIGN TEAM. THE CONTRACTOR SHALL STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING HAVE BEEN ADDRESSED. SUBMITTALS THAT DO NOT MEET THESE REQUIREMENTS WILL BE REJECTED.
A. THE SUBMITTAL IS EXPLICITLY REQUIRED PER THE STRUCTURAL DRAWINGS. ENGINEER WILL NOT REVIEW ITEMS THAT ARE NOT LISTED BELOW.
B. THE SHOP DRAWING IS BASED ON THE MOST CURRENT DRAWINGS, INCLUDING ALL ADDENDA AND SKETCHES
C. REVIEW COMMENTS FROM ANY PRIOR REVIEWS HAVE BEEN ADDRESSED.
D. ALL REVISIONS FROM ANY PREVIOUS SUBMISSIONS HAVE BEEN CLOUDED.
E. THE SUBMITTAL IS COMPLETE AND THE WORK HAS BEEN COORDINATED WITH ALL OTHER CONSTRUCTION TRADES.
F. THE SUBMITTAL DOES NOT INCLUDE ANY SUBSTITUTION REQUESTS.
3. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO STARTING CONSTRUCTION. THE LIST OF REQUIRED SHOP DRAWING SUBMITTALS IS AS FOLLOWS:
A. CONCRETE MIX DESIGNS FOR EACH APPLICATION
B. CONCRETE REINFORCEMENT STEEL
4. THE ENGINEER'S RESPONSIBILITIES RELATIVE TO SHOP DRAWING REVIEW ARE LIMITED TO REVIEW FOR DESIGN CONCEPT OF THE PROJECT AND INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS, SPECIFICALLY THE STRUCTURAL DRAWINGS AND THE STRUCTURAL PORTIONS OF THE PROJECT SPECIFICATIONS. THE ENGINEER WILL NOT REVIEW FOR DIMENSIONS, QUANTITIES, ETC. ADDITIONALLY, THE ENGINEER WILL NOT REVIEW SHOP DRAWINGS THAT ARE NOT EXPLICITLY LISTED ABOVE, OR ANY SUBMITTALS THAT ARE NOT RELATED TO THE MAIN BUILDING STRUCTURE.
5. SHOP DRAWING REVIEW NOTATIONS ARE DEFINED AS FOLLOWS:
A. "NO EXCEPTION": INDICATES THAT FABRICATION, MANUFACTURE, OR CONSTRUCTION MAY PROCEED.
B. "EXCEPTIONS AS NOTED": INDICATES THAT FABRICATION, MANUFACTURE, OR CONSTRUCTION MAY PROCEED PROVIDED THAT SUBMITTAL COMPLIES WITH ENGINEER'S NOTATIONS. IF, FOR ANY REASON, THE CONTRACTOR CANNOT COMPLY WITH THE ENGINEER'S NOTATIONS, THE SUBMITTAL SHALL BECOME "REJECTED" OR "DISAPPROVED" (REFER TO DEFINITION BELOW) AND THE CONTRACTOR SHALL IMMEDIATELY NOTIFY BOTH THE ARCHITECT AND ENGINEER.
C. "EXCEPTIONS AS NOTED RESUBMIT FILE COPY": INDICATES THAT FABRICATION, MANUFACTURE, OR CONSTRUCTION MAY PROCEED PROVIDED THAT SUBMITTAL COMPLIES WITH ENGINEER'S NOTATIONS. THE CONTRACTOR SHALL RETURN REVISED VERSIONS OF THE SUBMITTAL TO THE ARCHITECT AND ENGINEER FOR RECORD. IF, FOR ANY REASON, THE CONTRACTOR CANNOT COMPLY WITH THE ENGINEER'S NOTATIONS, THE SUBMITTAL SHALL BECOME "REJECTED" OR "DISAPPROVED" (REFER TO DEFINITION BELOW) AND THE CONTRACTOR SHALL IMMEDIATELY NOTIFY BOTH THE ARCHITECT AND ENGINEER.
D. "REJECTED": INDICATES THAT THE SUBMITTAL DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS AND THAT FABRICATION, MANUFACTURE, OR CONSTRUCTION SHALL NOT PROCEED. THE CONTRACTOR SHALL MAKE THE NECESSARY REVISIONS TO THE SUBMITTAL AND RESUBMIT IN A TIMELY FASHION SO AS TO NOT DELAY THE CONSTRUCTION SCHEDULE.

APPROVED
Department of Permitting Services
Permit # SEDIMENT-297245
Date 10/24/2025
Stamped By: Mark Etheridge

- E. "REVIEWED FOR INFORMATION": INDICATES THAT THE SUBMITTAL HAS BEEN RECEIVED BY THE ENGINEER FOR FILE ONLY. NO ADDITIONAL ACTION WILL BE TAKEN BY THE ENGINEER.
F. "PRE-ENGINEERED SYSTEMS SUBMITTAL": INDICATES THAT THE SUBMITTAL HAS BEEN REVIEWED FOR GENERAL CONFORMITY WITH THE DESIGN INTENT AND GENERAL COMPLIANCE WITH THE INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. COMMENTS AND/OR NOTATIONS MADE ON THE SUBMITTAL BY THE ENGINEER OF RECORD ARE INTENDED TO NOTIFY THE CONTRACTOR AND/OR THE RESPONSIBLE DESIGNER OF CONFLICTS BETWEEN THE SUBMITTAL AND THE DESIGN INTENT OR THE CONSTRUCTION DOCUMENTS. THESE COMMENTS AND/OR NOTATIONS DO NOT RELIEVE THE CONTRACTOR AND THE DESIGN ENGINEER OF SOLE RESPONSIBILITY FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS AND ALL APPLICABLE CODES AND LAWS. SOLE RESPONSIBILITY FOR CORRECT DESIGN, DETAILS, AND DIMENSIONS SHALL REMAIN WITH THE CONTRACTOR AND THE DESIGN ENGINEER.

Table with 6 columns: ABBREVIATION, DEFINITION, ABBREVIATION, DEFINITION, ABBREVIATION, DEFINITION. Lists various construction terms and their abbreviations.

CONCRETE DESIGN SHALL MEET THE REQUIREMENTS OF ACI 350, ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES, WITH FREEZING AND THAWING EXPOSURES. CONCRETE SHALL BE EITHER A TYPE 1L CEMENT CONFORMING TO ASTM-C-595, OR TYPE II OR IIA CEMENT WITH A 28 DAY COMPRESSIVE STRENGTH OF 4500 PSI FOR CAST IN PLACE AND 5000 PSI FOR PRE-CAST STRUCTURES. CONCRETE SHALL ALSO MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 420, MIX NO. 6.

STRUCTURAL CERTIFICATION

I hereby certify that the structural design of this stormwater management facility is in accordance with applicable codes and that the plan for this has been designed for specified loading(s) as indicated hereon.

Design/Engineer Signature: Jason T. Dreher
Date: 6/23/25
Printed Name: Jason T. Dreher
Registration Number: 36484
ACTIVE EARTH PRESSURE: 45 PSF/FT
VERTICAL SURCHARGE LOADING: 250 PSF
Design Loading



OWNER
SCG DEVELOPMENT
4205 SCONE BLVD, SUITE 640
VIENNA, VA 22182
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LANDSCAPE ARCHITECT
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Alexandria, VA 22314
Tel: 703.545.7784

STRUCTURAL ENGINEER
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MEP ENGINEER
Scot Engineering, Inc.
509 Germantown Pike Bldg 2
Lafayette Hill, PA 19444
Tel: 215.774.1423

NOBE II MULTIFAMILY
NORTH BETHESDA, MARYLAND

SWM STRUCTURAL NOTES

PROJECT NUMBER
22-062

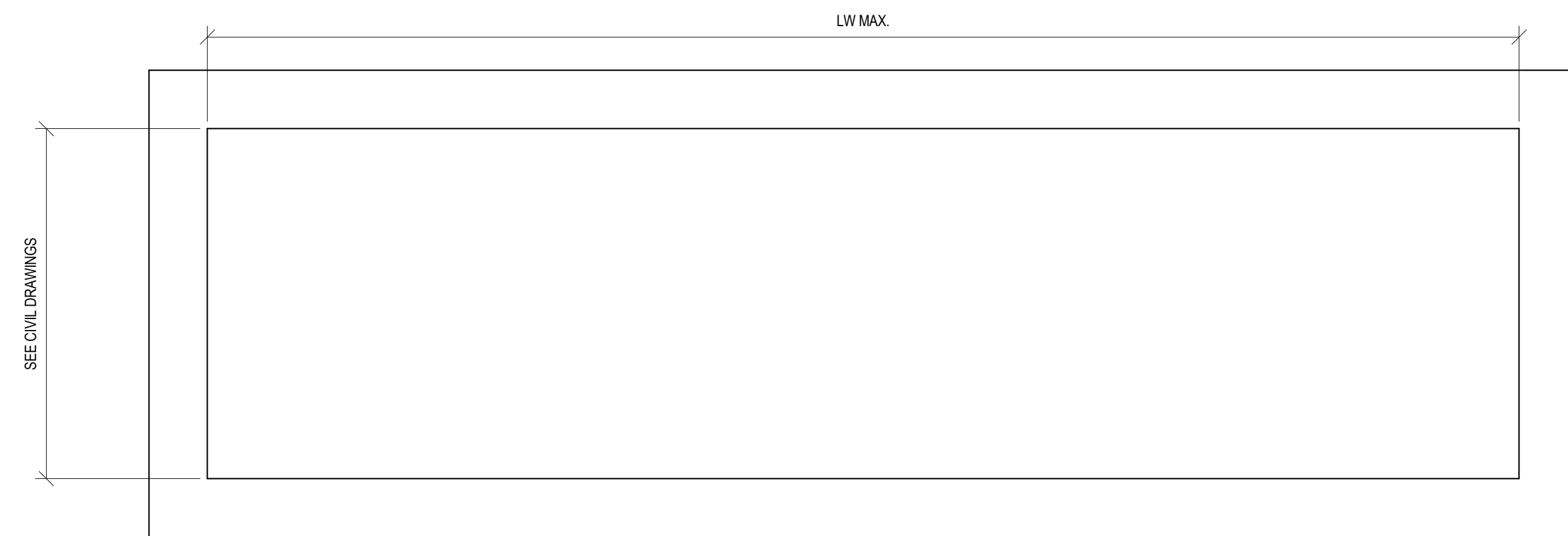
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KEY PLAN



DRAWING STAMP

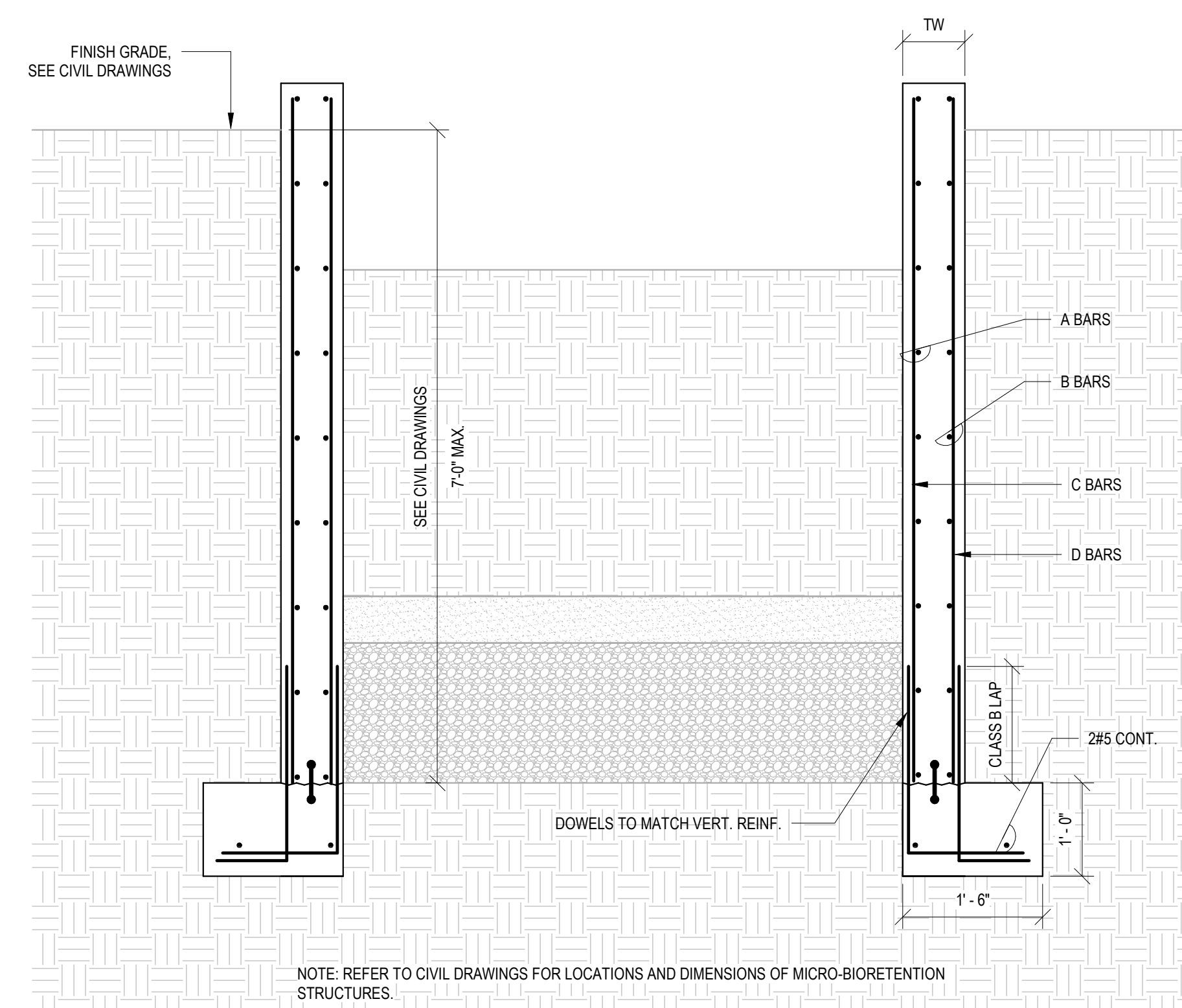
Sheet 15 of 16
SHEET NUMBER
S110



MICRO-BIORETENTION STRUCTURE PLAN

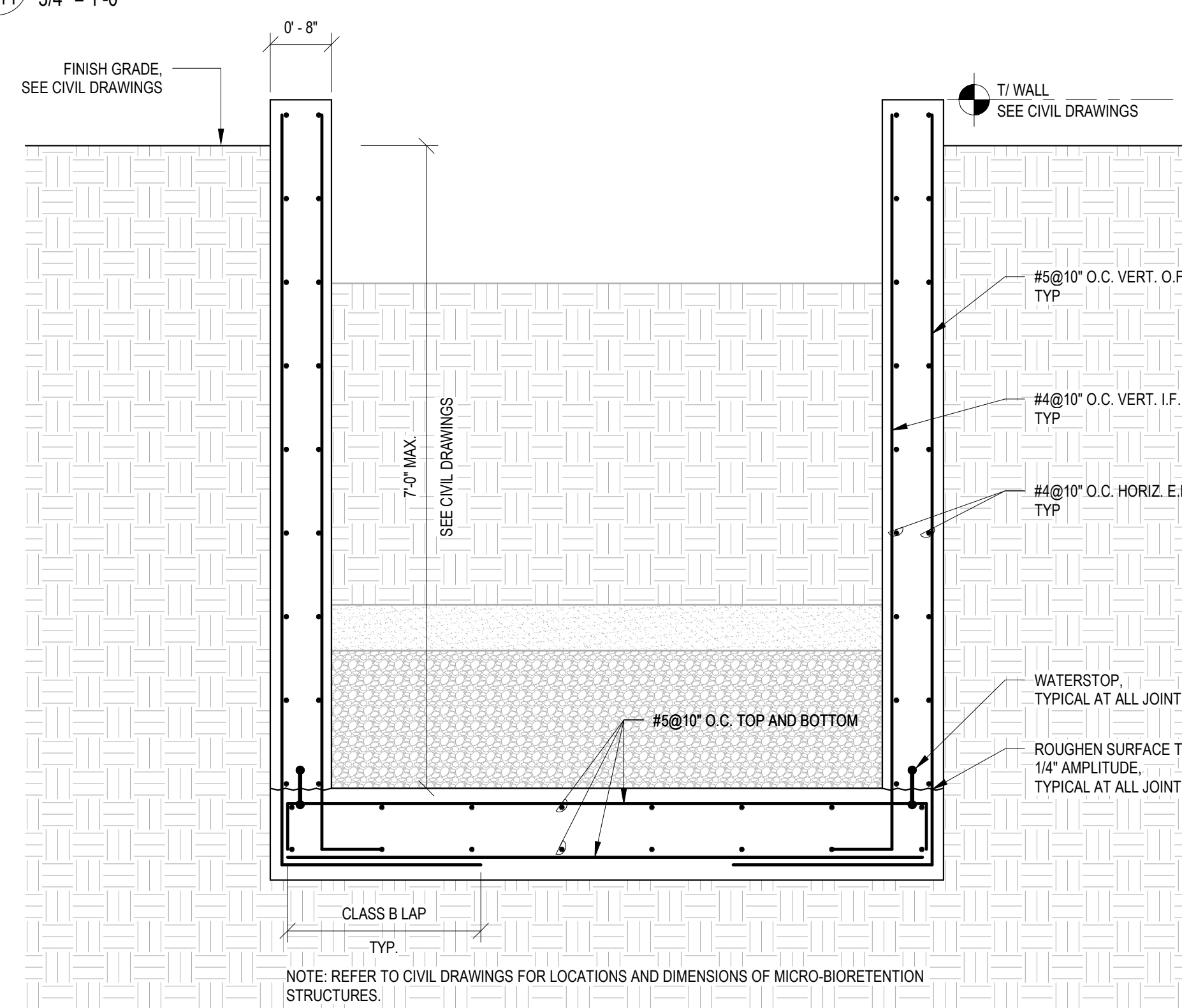
MICRO-BIORETENTION STRUCTURE SCHEDULE					
LW	TW	A	B	C	D
10'-0"	0'-8"	#4@8	#4@8	#4@12	#4@12
15'-0"	0'-10"	#5@8	#5@8	#4@12	#4@12
19'-6"	1'-0"	#6@8	#6@8	#5@12	#5@12
21'-0"	1'-0"	#6@6	#6@6	#5@12	#5@12

NOTE: WALLS FOR MICRO-BIORETENTION STRUCTURE MUST BE POURED WITH HORIZONTAL REBAR CONTINUOUS AND WITHOUT CONTROL JOINTS.



MICRO-BIORETENTION STRUCTURE SECTION

1
S111
TYPICAL MICRO-BIORETENTION STRUCTURE (MB)
3/4" = 1'-0"

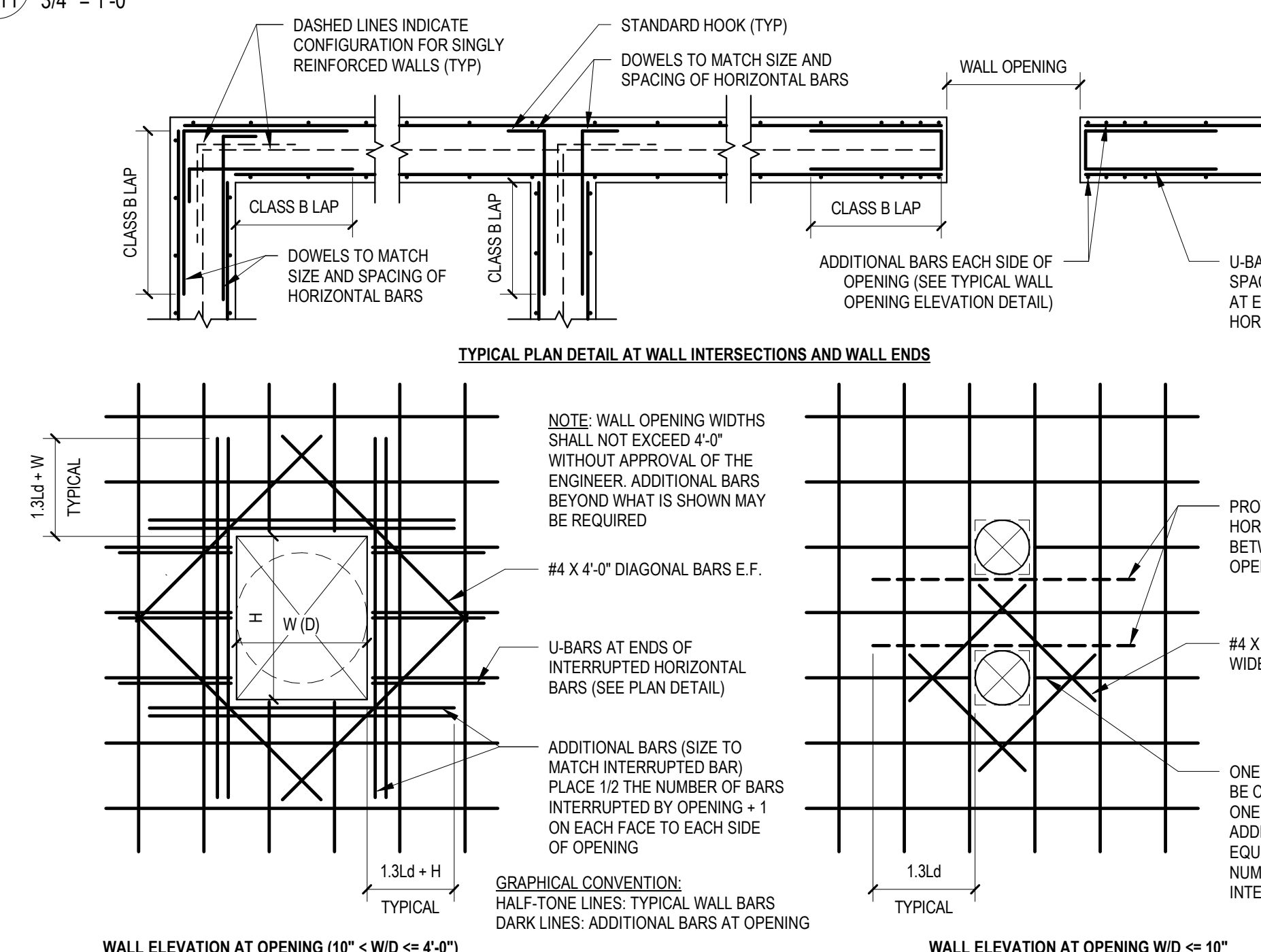


BAR SIZE		FC = 3,000 PSI		FC = 4,000 PSI		FC = 5,000 PSI		FC = 6,000 PSI		FC = 8,000 PSI	
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	A	22	32	17	25	19	28	17	25	16	23
	B	28	42	22	32	24	36	19	28	22	33
#4	A	29	43	22	33	25	37	19	29	22	33
	B	37	56	29	43	32	48	25	37	29	43
#5	A	36	54	28	41	31	47	24	36	28	42
	B	47	70	36	54	40	60	31	47	36	54
#6	A	43	64	33	50	37	56	29	43	33	50
	B	56	84	43	64	48	72	37	56	43	65
#7	A	63	94	48	72	54	81	42	63	49	73
	B	81	122	63	94	70	106	54	81	63	94
#8	A	72	107	55	82	62	93	48	72	55	83
	B	93	139	72	107	80	121	62	93	72	108
#9	A	81	121	62	93	70	105	54	81	63	94
	B	105	157	81	121	91	136	70	105	81	122
#10	A	91	136	70	105	79	118	61	91	70	105
	B	118	177	91	136	102	153	79	118	91	137
#11	A	101	151	78	116	87	131	67	101	78	117
	B	131	196	101	151	113	170	87	131	101	152

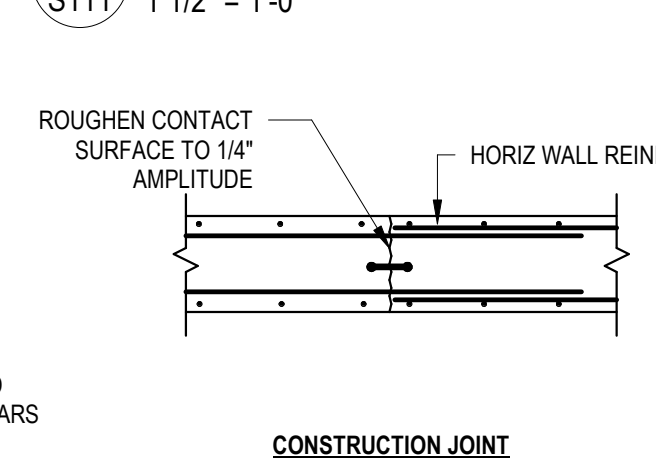
REBAR LAP SCHEDULE

- DETAIL NOTES:**
- TABULATED LAP SPlice LENGTHS ARE IN INCHES AND ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL-WEIGHT CONCRETE.
 - FOR LIGHTWEIGHT CONCRETE, MULTIPLY VALUES BY 1.30.
 - FOR EPOXY-COATED REINFORCING BARS, MULTIPLY VALUES BY 1.50.
 - TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS.
 - CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER-TO-CENTER SPACING OF THE BARS, ARE DEFINED AS:
 - BEAMS OR COLUMNS:
 - CASE 1: COVER AT LEAST d_b AND CENTER-TO-CENTER BAR SPACING AT LEAST $2d_b$
 - CASE 2: COVER LESS THAN d_b OR CENTER-TO-CENTER BAR SPACING LESS THAN $2d_b$
 - ALL OTHER CONDITIONS:
 - CASE 1: COVER AT LEAST d_b AND CENTER-TO-CENTER BAR SPACING AT LEAST $3d_b$
 - CASE 2: COVER LESS THAN d_b OR CENTER-TO-CENTER BAR SPACING LESS THAN $3d_b$
 - LAP SPlice LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS.
 - CLASS A LAP SPlice = $1.0 \cdot L_d$
 - CLASS B LAP SPlice = $1.3 \cdot L_d$
 - LAP SPlices OF #14 AND #18 BARS ARE NOT PERMITTED.
 - TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.

2
S111
TYPICAL MICRO-BIORETENTION STRUCTURE WITH BOTTOM SLAB (MBP-1)
3/4" = 1'-0"



3
S111
REBAR LAP SPlice SCHEDULE
1 1/2" = 1'-0"



4
S111
TYPICAL CONCRETE WALL DETAILS
1/2" = 1'-0"

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 Tel: 703.942.6610

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LANDSCAPE ARCHITECT
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 746 Walker Road
 Great Falls, VA 22066
 Tel: 202.669.1435

MEP ENGINEER
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 509 Germantown Pike Bldg 2
 Lafayette Hill, PA 19444
 Tel: 215.774.1423

NOBE II MULTIFAMILY

NORTH BETHESDA, MARYLAND

**SWM STRUCTURAL PLANS
 AND DETAILS**

PROJECT NUMBER

22-062

REV.	ISSUE	DATE
	Permit Set	10-22-24

KEY PLAN



DRAWING STAMP

SC/SWM - 16 of 16
 SHEET NUMBER

S111